BREVARD COUNTY FACILITIES ENGINEERING & CONSTRUCTION AUTHORIZATION TO INITIATE WORK

Construction Change	Directive		✓ Construction Contract Change Order					
Change Order to Purch	nase Order		✓ Guaranteed Maximum Price Realignment					
AUTH/CHANGE ORDER N	o: 04		CONTRACTOR: RUSH Construction, Inc.					
PROJECT TITLE: Space	View Park Im	provements	DATE: 05/14/202 1					
ORIGINAL CONTRACT AM	Τ;	\$ 1,798,899.00						
COST OF CURRENT CHAN	IGE:	\$ 230,706.26	PO Number:	4500108512				
CHANGE ORDERS APPRO	VED TO DATE:	\$ 20,373.53	W/O Number:					
NEW CONTRACT AMOUN	Γ:	\$ 2,049,978.79						
Fund: Co	ost Center:		G/L:	I/O Number:				
Owner Requested List Previously Approved		EError Omission	Unforeseen Cond	litions:				
Authorization #1	\$ 8,745.30		Authorization #2	\$ 23,400.00				
Authorization #3	\$ 32,104.53							
Authorization #5				Authorization #6				
RUSH Cons	truction, Inc	C.	Mark Luca:	S	05/14/21 Date			
		Date		as T	1/2/21			
CEG Architect/Engineer		Date	Engineer	1.	Date			
Parks and Recreation	n Finance	Date	Contracts Admin		Date			
Parks and Recreation	Butzlo Construction Man	6/7/2024 Date	County Manager		Date			
Parks and Recyeation	Department Direct	rtor 6/7/2021	Chair Board of Co	ounty Commissioners	Date			





RUSH Construction, Inc. 6285 Riverfront Center Blvd Titusville, Florida 32780 Phone: (321) 267-8100

Fax: (321) 267-9944

Project: 2025 - Brevard County - Spaceview Park 10 East Broad Street Titusville, Florida 32780

RUSH Construction, Inc.

Titusville, Florida 32780

6285 Riverfront Center Blvd

1 - Brevard County Space View Park

Mark Lucas (RUSH Construction, Inc.)

Prime Contract Potential Change Order #004: DOCK PILES SOUTH PIER

FROM:

CREATED BY:

CREATED DATE:

PRIME CONTRACT

CHANGE ORDER:

TO: **Brevard County**

2725 Judge Fran Jamison Way,

Melbourne, Florida 32940

CONTRACT:

Michael Butcher (Brevard County)

REQUEST RECEIVED FROM: STATUS: Pending - Proceeding

REFERENCE: **RFI 16**

No

LOCATION:

SCHEDULE IMPACT:

FIELD CHANGE:

PCO NUMBER/REVISION:

EXECUTED:

South Side Space View Park

64 days

No

ACCOUNTING METHOD:

PAID IN FULL: Nα

SIGNED CHANGE ORDER

RECEIVED DATE:

TOTAL AMOUNT:

\$230,706.26

Amount Based

5/14/2021

None

POTENTIAL CHANGE ORDER TITLE: DOCK PILES SOUTH PIER

CHANGE REASON: Existing Condition - Unforeseen

POTENTIAL CHANGE ORDER DESCRIPTION: (The Contract Is Changed As Follows)

CE #007 - DOCK PILES SETTLING SOUTH PIER SOUTH DOCK WOOD PILINGS HAVE SETTLED AFTER DRIVEN TO ELEVATION.

- RFI #16 submitted and the county and engineer requested soil borings.
- All dock work stopped pending engineering review and direction.
- Geotechnical Engineering Evaluation performed by Ardaman and Associates, Inc.
- Soil conditions were determined to be unacceptable for the dock design.
- A lateral load analysis was performed by Ardaman and Associates, Inc. to determine size of pilings required for soil conditions.
- · Longer piles were ordered after size determination.
- Marine Equipment was put on standby after RFI until longer piles were delivered.

ATTACHMENTS:

RFI 16.pdf , GC's L .pdf , GC's M.pdf , 2025 Ardaman & Associates 04.12.21 \$19594.75.pdf , RUSH Marine Proposal - Longer Piles 2021-0506.pdf , Ardaman Invoice C41888.pdf , Lateral Analysis Report 21-5223A.pdf , Lateral Analysis Proposal signed.pdf , Ardaman Proposal Signed 03-22-21.pdf

#	Budget Code	Description	Amount
1	13-02-01.S SPECIALTIES SUBCONTRACTOR.SUBCONTRACT	Additional pilings, bracing and standby costs	\$148,724.00
2	01-90-07.M SOIL TEST.MATERIAL	Soil borings for South Dock	\$19,594.75
3	01-90-23.M DESIGN SERVICES -STRUCTURAL.MATERIAL	Lateral Load Analysis	\$1,500.00
4	01-30-02.G CONTRACTOR FEE.GENERAL	General Conditions "L"	\$4,530.00
5	01-30-40.L SUPERVISION.LABOR	Superintendent	\$16,695.00
6	01-30-36.L SAFETY / QC REP.LABOR	Special Safety and QC Requirement	\$6,720.00



#	Budget Code	Description	Amount
7	01-30-92.M SMALL TOOLS & EXPENDABLES.MATERIAL	Small Tools and Expendables	\$300.00
8	01-51-05.M TELEPHONE USAGE (LAND/CELL).MATERIAL	Field Telephone	\$200.00
9	01-51-21.M DRINKING WATER.MATERIAL	Ice and Water	\$225.00
10	01-51-24.M TEMPORARY SANITARY FACILITIES.MATERIAL	Temporary Toilet	\$250.00
11	01-30-50.L LABOR BURDEN.LABOR	PT and I	\$7,416.00
12	01-30-18.M GENERAL LIABILITY INSURANCE.MATERIAL	GL Insurance rate 0.5%	\$1,030.77
13	01-30-02.G CONTRACTOR FEE.GENERAL	Overhead at 5%	\$10,359.28
14	01-30-02.G CONTRACTOR FEE.GENERAL	Profit 5%	\$10,877.24
15	01-30-22.M CONTRACT BOND.MATERIAL	Bond 1%	\$2,284.22
		Grand Total:	\$230,706.26

David Tom (Construction Engineering Group)

2651 W. Eau Gallie Blvd. Suite A Melbourne, Florida 32935

Brevard County

2725 Judge Fran Jamison Way, Melbourne, Florida 32940

RUSH Construction, Inc.

6285 Riverfront Center Blvd Titusville, Florida 32780

SIGNATURE

SIGNATURE

DATE

SIGNATURE

DATE

RUSH Construction, Inc ATTACHMENT "L" COST ESTIMATE BREAKDOWN NO. 1A *Required per Specs

PAGE 1 OF 2

TITLE: Space View Park BC FEMA Continuing Services

TRADE: ALLOWABLE OVERHEAD

0 SF TOTAL: \$

5,510

LOCATION: Titusville

SCHEDULE: CONTRACT: 8 DAYS
WORK: 2 MOS

90 WKS

					WORK:	2	MOS		9.0	WKS	
LINE	ITEM			MATER.	COST	L	ABOR (COST	OTHER DIR.	LINE	1
NO.		UNIT	QTY.	UNIT	TOTAL	HRS.	RATE	TOTAL	COST	TOTAL	
1	PROJECT MANAGER	WKS	4.0	175.00	700	64	45.00	2,880		3,580	01-30-03
	16 hours per week										General L
2	ASST PM	WKS	=	551	3	190	42.00	90	-	565	
3	EXECUTIVE LABOR COST	wks	₹.	0.72		2.50	42.00			286	
	FOTIMATOR			1415							
4	ESTIMATOR	wks	~	79	8		42.00	24		877.7	
5	OFFICE SECRETARY	_{wks}	2	14	9	(S)	0.00	2.7			
"	OTTIGE GEGINETATO	*****			_		0.00	-		•	
6	SCHEDULER	wks	*	241	=	:46	45.00	22		942	
7	AUTOMOBILES/ TRUCKS	MOS	2.0	300.00	600	(+0)	0.00	:41		600	01-10-03
											General L
8	FUEL & MAINTENANCE	MOS	2.0	175.00	350	(#P)	0.00	54	× 1	350	01-10-03
1 1											General L
9	PM TRAVEL	WKS			2	180	0.00	-		88	
10	WARRANTY EXPENSE	LOT			*		42.00	90		· · ·	
	(incl. equip. I.D. tags)										
11	POSTAGE				_		0.00				
1 '' 1	POSTAGE	MOS	-	(Ga)	2	-	0.00	1		3. *!	
12	OFFICE SUPPLIES	LOT			·		0.00	_			
'*	OFFICE SOFFEES			3.97	_ ^		0.00				
13	OFFICE PHONES	Mos		O = €		· · · ·	0.00				
'-				l has			0.00				
14	MISC FEES	LOT		185		: =:::	0.00				
	SUB TOTAL	S PAG	SES 1		\$ 1,650			\$ 2,880	\$ -	\$ 4,530	1
		SALES	TAX %	7	116	PT&I %	30	864	4	980	Ē.
				TOTALS	\$ 1,766			\$ 3,744	\$ - RIFIED TOTAI	\$ 5,510 \$ 5,510	

^{** 4/24/2020}

RUSH Construction, Inc ATTACHMENT "M" COST ESTIMATE BREAKDOWN NO.

PAGE 1 OF 2

TITLE:Space View Park BC FEMA Continuing Services

*Required per Specs TRADE: GENERAL CONDITIONS

_0 SF TOTAL: \$

31,121

LOCATION: Titusville

SCHEDULE: CONTRACT:

WORK: 2 MOS 9 WKS

LINIE	TEM	_			WURK:		MOS			WKS	
LINE	ITEM	LINUT	OTV	MATER.	COST			OST	OTHER DIR.	LINE	ľ
NO.	SUPERVISION	UNIT	QTY.	UNIT	TOTAL	HRS.	RATE	TOTAL	COST	TOTAL	
	SUPERVISION	WKS	9	175,00	1,575	360	42.00	15,120		16,695	01-30-40
2	ASST, SUPERINT.	wks	0	2	3	<u> </u>	0.00	=			
3	SPECIAL QC REQ.	WKS	4	*	œ	160	42.00	6,720	:21	6,720	01-30-36
4	SSHO MANAGER	wks	0	*	(*)		0.00			9	
5	ESCORT	wks	0	5	250	E1	0.00		3	-	
6	SAFETY DIRECTOR	wks	0	=	14-	2	42.00	8	3	3	
7	CQC MANAGER	wks			(96)	100	42.00	-		¥	
8	ASST QC REP	wks		2	100	, le:	0,00		Ge .		
9	CLERK	wks	0	1.2	14,	148	35.00	3	Ē	,	
10	SCHEDULER	wks		je.	280	: 60	0.00	-	22	ş	
11	OFFICE/STORAGE UNIT	MOS UN	0	200.00	(25)	: #	35.00		:=	-	RM
12	STOR TRAILERS	MOS	0		0.54	450	35,00	=	2		
13	FIELD TELEPHONE	MOS	2	100.00	200	720	0,00	3	ē	200	01-51-05
14	POWER	MOS	2	160	020	141	0.00		5-	i i	
15	TEMP. WATER	LOT	0	0.00		1000	0.00	*	3-		
16	ICE & WATER	WKS	9	25.00	225	(#)	0.00	*	(=	225	01-51-21
17	TEMP. TOILETS	MOS	2	125.00	250	05	0.00	*	æ	250	01-51-24
18	DROP TANK	MOS		1/7%	973	150	0.00	5	25		
19	DUMPSTERS	UN EA	2	100	(S)	\\\	0.00	€	8	3	
20	WEEKLY CLEANUP	wks	0	(e)	**	y(#)	35,00	2		-	
21	FINAL CLEANUP	SF	0		82	5.00	35.00	*	:=	*	
22	SURVEYS/C, PNTS.	LOT	1	~		18	0.00	ê	ā	ē	
23	LAYOUT / B. BOARDS	LOT	0	::#:	∞	825	0.00	¥	5	9	
24	PROJECT SIGN	EA	1	((#)	·	:(#)	35.00		:-	*	
25	OWNERS TESTING	LOT	0	150		(\ E \$	0.00		27	=	
26	TESTING	LOT	1	富		720	0.00	8	ž.		None planned
	SUB TOTA	I S THI	SPAGE		\$ 2,250			\$ 21,840	\$ -	\$ 24.090	1
					2,200	E	73	21,070		4 27,030	1

RUSH FORM 102 (1/98): EX

RUSH Construction, Inc COST ESTIMATE BREAKDOWN NO. 1 PAGE 2 OF 2

ATTACHMENT "M" TITLE:Space View Park BC FEMA Continuing Services

TRADE: GENERAL CONDITIONS

TOTAL: \$ 31,121

24,090

VERIFIED TOTAL \$

LOCATION: Titusville

SCHEDULE: CONTRACT:

U											
LINE	ITEM			MATER.	COST	L	ABOR CO)ST	OTHER DIR.	LINE	1
NO.		UNIT	QTY.	UNIT	TOTAL	HRS.	RATE	TOTAL	COST	TOTAL	
27	TEMP. FENCING	LF	0	4.50		1927	35.00				RM
28	SITE SECURITY	wks	0	3 .	548	920	35.00	2	=	=	
29	REPRODUCTION	LOT	0	38:	:	(*)	0.00	*		*	
30	SMALL TOOLS &	MOS	2	150.00	300	353	0.00			300	01-30-92

31	SAFETY EQUIPMENT OFFICE TRAILER FURNITURE	LOT	0		-		0.00	2	***	7 4 11	
32	OFFICE TRAILER EQUIPMENT	LOT	0	÷	2	2	0.00	ing.			RM
33	AS-BUILT DWGS	LOT	0		-:		42.00			*	
34	(Red-lines) AS-BUILT DWGS- Elect,	LOT	0			8	0.00		127	(4)	
35	CPM SCHEDULE	MOS	0	₽	2	2	0.00	3	8.	520	
36	P.E. CERT PERMITS	LOT	0	-		¥	0.00	2	(2 2 ()	541	
37	MISC SITE RESTORE	LOT	0	*	=	5 ;	0.00		901	:=:	
38	GIS SURVEY (With Surveys/C. Points)	LOT	0	2		÷	0.00	ŝ	570		
39	HURRICANE PREP	LOT	0		*	=	0.00	2	(2)	HIV:	
44	PARTNERING (Design Review Mtgs)	LOT	0	5		8	0.00	*	:=::	(4)	
45	CLOSE OUT EXPENSE	LOT	0	£	18	- 8	42.00		250	:60	
47	PAINT JOB TRAILER	LOT	0	24		20	0.00	5	57	3	
48	SAFETY COMPL.	LOT	0	*			0.00		(#)	(#.)	
49	COLOR BOARDS	LOT	0		7.54	=	0.00	-	· .	· ·	
50	I.D. BADGES	LOT	0		N#1	125	0.00		3 (3.	
51	EQUIP LAYOUT DWGS	LOT	0	*	:e	36	0.00		147	41	
52	PROGRESS PHOTOS	MOS	0	18	(#E		0.00			341	
53	BUILDERS RISK	LOT	4		(6)	16	0.00	=	577	(E)	
54	PERMIT ALLOWANCE	LOT	1	180	N#F	185	0.00	¥	3	4 1	
55	P&P BOND	LOT	0	590	X e s	18	0.00	*	90		
56	G/L INSURANCE	LOT	0		95	1.50	0.00		a		
57	ACCESS ROAD	LOT	0	24	S\$	YES	0.00	2	9	31	
	SUB TOTAL		ES1&2 TAX %	7 TOTALS	\$ 2,550 179 \$ 2,729	PT&I %	30	\$ 21,840 6,552 \$ 28,392		\$ 24,390 6,731 \$ 31,121	
**	4/24/2020			TOTALS	Ψ 2,129				RIFIED TOTAL		

** 4/24/2020

Printed On: May 12, 2021 01:35 PM EDT



10 East Broad Street Titusville, Florida 32780

RFI #16: SOUTH DOCK PILE SETTLING

Status Closed on 05/12/21

To Michael Butcher (Brevard County)

Pete Petterson (Applied Technology &

Management)

David Tom (Construction Engineering Group)

From

Mark Lucas (RUSH Construction, Inc.)

6285 Vector Space Blvd.

Titusville, Florida 32780

Date Initiated

Mar 12, 2021

Due Date

Mar 15, 2021

Location

Project Stage

Course of Construction

Cost Impact

Yes (Unknown)

Schedule Impact

Yes (Unknown)

Spec Section

Cost Code

Drawing Number

S1.0, S2.0, S5.0

Reference

Linked Drawings

Received From

Matt Tate (RUSH Marine, LLC)

Copies To

Ralph Bush (RUSH Construction, Inc.), Tony Landry (RUSH Marine, LLC), Joe Pellegrino (RUSH Construction, Inc.), Matt Tate

(RUSH Marine, LLC)

Activity

Question

Question from Mark Lucas RUSH Construction, Inc. on Friday, Mar 12, 2021 at 01:28 PM EST

As per our telephone discussion, during construction of the south dock walkway March 9, 2021 we discovered some of the piles had sunk slightly from their installed elevation. On March 10,2021 we discovered they had sunk significantly more. On March 11, 2021 we made some adjustments and raised the framing back to design elevation. But we are still noticing some settling this day. The problem appears to be limited to the south dock east end of walkway span. See attached drawing which identifies the pile locations, lengths and as-built deck elevations taken on March 12, 2021. Note: The actual pile embedment depths exceed the design requirement of 6' of embedment and in some places is over double the requirement. See also, attached photo of dock taken on March 11, 2021. Please review and advise what course of action we should take.

Attachments

RFI 16 attachment.pdf, RFI 16 photo attachment.pdf

Official Response

Response from David Tom Construction Engineering Group on Monday, Mar 15, 2021 at 02:23 PM EDT

I spoke to Ardaman and we think that 3-borings located adjacent to the area of the pier that has dropped at depths between 30'-40' should be sufficient for Ardaman to make their recommendations.

Attachments

RFI 16 Borings.pdf

Official Response

Response from Michael Butcher Brevard County on Monday, Mar 15, 2021 at 07:44 AM EDT

Brevard County has contacted Ardaman and requested geotechnical work or soil borings. BCPR request that the engineer designate what areas and what depths they would like the borings taken. Ther has been no guarantee from Ardaman on a time frame but hopefully Monday 3/15 we will have a better idea.

	Ą	As-Built Deck Difference	Difference	As-Built Deck Difference	Difference	
	Bent	日	inch	EL	inch	
	00	5.86	0.48	5.81	1.08	
	6	5.72	2.16	5.74	1.92	
	10	5.69	2.52	5.68	2.64	
•	11	5.88	0.24	5.80	1.20	
•	12	5.90	00.00	5.87	0.36	
	13	5,84	0.72	5.87	0.36	
	14	5.77	1.56	5.84	0.72	
0" 10' - 0" 10' - 0" 10' - 0" 10' - 0" 10' - 0" 10' - 0"	15	5.54	4.32	5.54	4.32	10' - 0" 11
	16	5.27	7.56	5.21	8.28	
	17	5.20	8.40	5.19	8.52	
	18	5.14	9.12	5.12	9.36	
	19	5.73	2,04	5.79	1.32	
A A A A A	THE STATE OF THE S	W.	W.	The state of the s	The state of the s	
1					11	
	m* }	EMA	**		Ba Ba	3
- ,9	/					
Bents 1 thru 10 25' piles 200'-	0			Bents 11 thru 30' piles	thru 19 les	



March 22, 2021 Proposal File No. 2123-048

Rush Construction 6285 Riverfront Center Boulevard Titusville, Florida 32780

Attention:

Mr. Mark Lucas

Subject:

Proposal for Subsurface Soil Exploration and

Geotechnical Engineering Evaluation

Brevard County Parks & Recreation Dock Replacement at

Space View Park Titusville, Florida

Dear Mr. Lucas:

As requested by Mr. Michael Butcher with the Brevard County Parks & Recreation Department, we are pleased to present this proposal for conducting a subsurface soil exploration and geotechnical engineering evaluation for the subject project. Based on our review of information provided by Mr. Butcher, the proposed project consists of the replacement of one wooden dock located at the Space View Park site which was damaged during a previous storm. The subject dock to be replaced is located within the Indian River adjacent to the park facility.

The length of the new dock will be approximately 220 feet. Preliminarily, it is estimated that the wooden pilings for the new dock will be installed to depths of no more than approximately 12 to 20 feet below the mud line within the river.

The scope of our work will include determining if the soil characteristics are suitable to construct the new dock piles. In addition, we will estimate axial pile capacities. Our proposed scope of services and the associated costs are presented below.

FIELD EXPLORATION

The field exploration program would consist of drilling four Standard Penetration Test (SPT) borings within the river in close proximity to the proposed dock location. Barge-mounted drilling equipment will be utilized. Each boring would be drilled to a depth of 40 feet below the mudline in the bottom of the river. We have assumed a depth from barge level to the mudline on the bottom of the river of no more than 10 feet.

The SPT borings will be drilled using a procedure similar to the Standard Penetration Test outlined in ASTM D-1586. To facilitate drilling within the river, solid casing will be set from the barge level to several feet below the mudline on the bottom of the river. The borings will be sampled at 18-inch intervals to 10 feet deep and at 5-foot intervals below 10 feet. Each sample will be removed from the sampler in the field and then examined and visually classified by our crew chief.

Representative portions will be sealed and packaged for transportation to our laboratory for further analysis as required.

LABORATORY PROGRAM

Routine laboratory visual classification will be performed along with specific classification tests deemed necessary (i.e., percent fines).

ENGINEERING ANALYSIS AND REPORT

Engineering analysis of all data obtained will be made to evaluate general subsurface conditions and to develop engineering recommendations to guide foundation support for the replacement dock. We will provide estimated axial compressive and uplift capacities for up to two diameters of round timber piles for support of the new dock. Our analyses do not include lateral load analysis.

Our recommendations for the dock piles, together with data developed during the exploration, will be submitted in a written report upon conclusion of the study.

COST ESTIMATE

The anticipated cost for the requested scope of services will not exceed **\$18,416.00** without prior approval from the client.

CLOSURE

This proposal is subject to the following conditions: (1) access to boring locations is to be readily available to our barge-mounted drilling equipment, (2) an existing boat ramp that is suitable for launching of the barge is located within close proximity of the project sites, (3) the proposed number of borings and the boring depths will be adequate, (4) undisturbed samples and consolidation tests on fine grained soils are not budgeted into the total cost, (5) Ardaman & Associates will not take responsibility for damages to underground or underwater structures and/or services that are not located by Sunshine One-Call, and (6) exploration or evaluation of the environmental (ecological or hazardous/toxic material related) condition of the site and subsurface is not included.

We appreciate the opportunity to submit this proposal and look forward to working with you on this project. If this proposal meets with your approval, please indicate your acceptance by signing and returning the attached Proposal/Project Acceptance Form. Please call if you have any questions or require additional information.

Very truly yours,

ARDAMAN & ASSOCIATES, INC.

Jason P. Manning, P.E.

Branch Manager

JPM/dk



PROPOSAL/PROJECT ACCEPTANCE AND AGREEMENT

PROJECT INFORMATION: Project Name Space View Park Dock Replacement Project Location Titusville, Florida Proposal Number and Date 2123-048; dated March 22, 2021 Description of Services Subsurface Soil Exploration Estimated Fee \$18,416.00 PROPERTY OWNER IDENTIFICATION: Name_____ Property Identification Number_____ City/State______ Zip Code______ Phone______ Attention______ Title_____ SPECIAL INSTRUCTIONS: **PAYMENT TERMS:** Payment shall be due within 30 days after date of each periodic invoice. Interest at the rate of 18% per annum (or the highest rate allowable by law) shall accrue on all amounts not paid within 30 days after date of invoice. All attorney fees and expenses associated with collection of past due invoices will be paid by Client. Failure to timely pay any invoice shall constitute a waiver of any and all claims arising from or related to Ardaman & Associates, Inc.'s services, including but not limited to the services described in this Proposal. PROPOSAL ACCEPTANCE: By accepting this Proposal, the Terms and Conditions of this Proposal, including the Terms on this page, and Ardaman & Associates, Inc.'s General Conditions appearing on the following page of this Proposal, are incorporated herein by reference. In the event this Proposal Acceptance was received by facsimile, Client hereby confirms that the above described Proposal, the Terms and Conditions of this Proposal, including the Terms on this page, and Ardaman & Associates, Inc.'s General Conditions have been made available and are incorporated in this agreement. Accepted this 22 14 day of 18ch , 2021 (Print or type individual, firm or corporate body name) (Signature of authorized representative)

Mok Lucas Rush RM.

(Print or type name of authorized representative and title)

Revision 2017 – FL

Subsurface Soil Exploration and Geotechnical Engineering Evaluation Dock Repairs Project Space View Park Titusville, Florida



CORPORATE HEADQUARTERS

8008 S. Orange Avenue, Orlando, FL 32809 - Phone: (407) 855-3860 Fax: (407) 859-8121

Branch Office Locations

Florida: Bartow, Cocoa, Fort Myers, Miami, Orlando, Port St. Lucie, Sarasota, Tallahassee, Tampa, West Palm Beach Louisiana: Baton Rouge, Monroe, New Orleans, Shreveport

MEMBERS:

ASTM International
American Concrete Institute
Geoprofessional Business Association
Society of American Military Engineers
American Council of Engineering Companies



April 7, 2021 File No. 21-23-5223A

Rush Construction 6285 Riverfront Center Boulevard Titusville, Florida 32780

Attention: Mr. Ma

Mr. Mark Lucas

Subject:

Subsurface Soil Exploration and

Geotechnical Engineering Evaluation

Dock Repairs Project Space View Park Titusville, Florida

Dear Mr. Lucas:

As requested, we have completed a subsurface soil exploration and geotechnical engineering evaluation for the subject project. The purposes of performing this exploration were to evaluate the general subsurface conditions within the proposed dock repair areas and to provide recommendations for foundation support for the dock structure. This report documents our findings and presents our engineering recommendations.

SITE LOCATION AND DESCRIPTION

The project site is the existing Space View Park facility located at the east end of Broad Street in Titusville, Brevard County, Florida (Section 3, Township 22 South, Range 35 East). The general site location is shown superimposed on the Titusville, Florida USGS quadrangle map presented as the Site Location Map, Figure 1.

The field exploration program documented in this report was performed within the Indian River adjacent to the new dock that is currently under construction on the east portion of the park facility. This new dock is being constructed to replace the previous dock that suffered significant storm damage.

The new dock that is under construction is a wooden dock that is approximately 220 feet in length. Eight-inch tip diameter timber pilings were previously installed at the project site for support of the dock structure. Information provided by Rush Construction indicates that many of the timber pilings that were installed for the segment of the dock beginning at Bent No. 8 and extending to near the east end of the dock have experienced significant settlement. The pilings in the area of Bent No. 8 to the east end of the dock are reportedly 20 to 35 feet in length, with approximately 10 to 18.5 feet of embedment below the mud line in the river. According to Rush Construction representatives, the existing timber pilings from Bent No. 8 to the west end of the dock have not experienced any notable settlement.

It is our understanding that the client is evaluating remedial measures to address the settlement of the pilings from Bent No. 8 to the east end of the dock. These remedial measures may include extending the existing pilings deeper and/or adding new longer pilings. Larger diameter pilings may be utilized.

FIELD EXPLORATION PROGRAM

SPT Borings

The field exploration program included performing four Standard Penetration Test (SPT) borings within the Indian River adjacent to the new dock that is under construction using barge-mounted drilling equipment. It is noted that Borings B-1 through B-3 were drilled in close proximity to pilings that have reportedly settled (east of Bent No. 8). Boring B-4 was drilled within an area where no obvious settlement of dock pilings has been noted by the Client (west of Bent No. 8). The borings were advanced to depths ranging from 40 to 60 feet below the mud line in the bottom of the river using the methodology outlined in ASTM D-1586. A summary of this field procedure is included in Appendix I. To facilitate drilling within the river, solid casing was set from barge level to several feet below the river bottom.

Split-spoon soil samples recovered during performance of the borings were visually classified in the field and representative portions of the samples were transported to our laboratory in sealed sample jars.

Test Locations

The approximate locations of the borings are schematically illustrated on a site plan shown on Figure 2. These locations were determined in the field by estimating distances from existing site features and should be considered accurate only to the degree implied by the method of measurement used.

LABORATORY PROGRAM

Representative soil samples obtained during our field sampling operation were packaged and transferred to our laboratory for further visual examination and classification. The soil samples were visually classified in general accordance with the Unified Soil Classification System (ASTM D-2488). The resulting soil descriptions are shown on the soil boring profiles presented in Appendix II.

GENERAL SUBSURFACE CONDITIONS

General Soil Profile

The results of the field exploration and laboratory programs are graphically summarized on the soil boring profiles presented in Appendix II. The stratification of the boring profiles represents our interpretation of the field boring logs and the results of laboratory examinations of the recovered samples. The stratification lines represent the approximate boundary between soil types. The actual transitions may be more gradual than implied.

The results of the SPT borings indicate the following general soil profile:

Approximate Depth Below River Bottom (feet)	Description (Unified Soil Classification)
0 to 18.5	Very soft organic muck/silt (OH, ML) and very loose to loose fine sand (SP), fine sand with silt (SP-SM), silty fine sand (SM), and clayey fine sand (SC)
18.5 to 52.5	Very loose to medium dense fine sand (SP), fine sand with silt (SP-SM), silty fine sand (SM), and clayey fine sand (SC). Also soft clay (CL/CH) in Boring B-3
52.5 to 60	Medium dense fine sand with silt (SP-SM) and silty fine sand (SM)

Various amounts of shell were encountered in the samples collected from the borings. The above soil profile is outlined in general terms only. Please refer to Appendix II for soil profile details.

River Water Depths

As indicated on the soil boring profiles in Appendix II, the river water depths approximated at the locations of Borings B-1 through B-4 ranged from 5 to 8 feet.

ENGINEERING EVALUATION AND RECOMMENDATIONS

Based on information provided by the Client, the existing 8-inch tip diameter timber pilings from Bent No. 8 to near the east end of the dock structure have experienced significant settlement. These exiting pilings reportedly have approximately 10 to 18.5 feet of embedment below the mudline in the river. It is likely that the settlement of the pilings that has occurred is due to the pile tips being located in the very soft organic muck/silt soils and/or the pilings not being installed deep enough below the bottom of the very soft soils.

It is our understanding that the Client is evaluating remedial measures to address the settlement of the pilings from Bent No. 8 to the east end of the dock. These remedial measures may include extending the existing pilings deeper and/or adding new longer pilings. Larger diameter pilings may be utilized.

The following are our recommendations for installation of new pilings from Bent No. 8 to near the east end of the dock structure based on the existing soil conditions. The recommendations are made as a guide for the design engineer, parts of which should be incorporated into the project's specifications.

Allowable Axial Compressive Capacity Analysis

Static axial compressive pile capacities were estimated following procedures utilizing SPT N-values obtained from Borings B-2 and B-3. Specifically, the computer software program "FB-Deep", which utilizes procedures developed by Dr. Schmertman originally outlined in

Research Bulletin No. 121-B dated September, 1967, was used. Estimated compressive pile capacities for individual 8-, 10-, and 12-inch tip diameter timber piles for the support of the dock structure are provided in the following table.

Depth Below	Allowable A	Axial Compressi (tons)	ve Capacity	
Mudline/River Bottom (ft)	8-Inch Diameter Tip Piles	10-Inch Diameter Tip Piles	12-Inch Diameter Tip Piles	
40	5	6	8	
45	6	8	10	
50	8	11	13	
55	11	14	18	

It is noted that scour has not been considered in the pile capacity analyses presented above. If scour is to be considered, we should be provided with the estimated amount of scour. The pile capacities presented above incorporate a factor of safety of 2 for both the side friction and the mobilized end bearing. We note that the estimated axial capacities are for individual piles. Pile spacing should be at least 3.0 pile diameters center to center to prevent reduced capacities caused by pile to pile interaction.

Soils containing a significant amount of shell (identified as "and shell" on the soil boring profiles in Appendix II) were encountered in the SPT borings at depths ranging from approximately 17.5 to 57.5 feet below the mud line in the bottom of the river. These soils may be difficult to penetrate and the contractor should ensure that the pile driving equipment utilized is adequate to achieve the required pile depths. The actual pile lengths will be determined during driving. Uplift capacities on the order of 50 percent of the vertical capacity should be used in design.

It is noted that our scope of work did not include a lateral capacity analysis for the timber piles. Once a final pile configuration is selected, a lateral capacity analysis should be performed by Ardaman & Associates, Inc. to further evaluate pile capacity. The pile lengths may need to be adjusted following the completion of the lateral load analysis.

Pile Installation

As specified in Revised Florida Department of Transportation (FDOT) Specification Section 455, piles should not be driven beyond practical refusal.

Piles should be driven with a steam, air, or diesel hammer delivering not less than the appropriate minimum rated energy per blow. At all times, the hammer should be operated at the chamber pressure and speed recommended by the manufacturer. Pile driving should be as continuous an operation as possible and should proceed without stopping over the last 5 feet of penetration. The piles should be driven with a cap block to prevent brooming.

During driving, pile driving records should be kept for each pile detailing pertinent information such as the pile type, pile length, date driven and blow count per foot. The capacity of each pile

should be reviewed based on its final tip elevation and driving record. For this project, we recommend that pile acceptance be based on driving below the maximum allowable pile tip elevation and attaining capacity based on an accepted dynamic formula (ie, Gates, Modified ENR, etc.). We note that the ENR formula incorporates a relatively high safety factor for axial capacity analysis. The safety factor could be reduced using a static load test.

A representative of Ardaman & Associates, Inc. should monitor the installation of the piles. Their duties should include, but not be limited to, the following:

- 1. Keep an accurate record of pile installation and driving procedures.
- 2. Verify that all piles are installed to the proper driving resistance and to a depth indicating that the piles were driven into the desired bearing formation.
- 3. Inspect the piles prior to installation for defects and confirm that the piles are not damaged during installation.
- 4. Confirm that the pile driving equipment is operating properly.

QUALITY ASSURANCE

We recommend establishing a comprehensive quality assurance program to verify that all foundation construction is conducted in accordance with the appropriate plans and specifications. Materials testing and inspection services should be provided by Ardaman & Associates. We recommend inspecting and testing the construction materials for the foundations and other structural components. The installation of the pile foundations for support of the dock structure should be monitored full-time by a representative of Ardaman & Associates.

CLOSURE

The analyses and recommendations submitted herein are based on the data obtained from the soil borings presented on Figure 2 and in Appendix II. This report does not reflect any variations which may occur adjacent to or between the borings. The nature and extent of the variations between the boring locations may not become evident until during construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations presented in this report after performing on-site observations during the construction period and noting the characteristics of the variations. This study does not include an evaluation of the environmental (ecological or hazardous/toxic material related) condition of the site and subsurface.

This report has been prepared for the exclusive use of Rush Construction in accordance with generally accepted geotechnical engineering practices. In the event any changes occur in the design, nature, or location of the proposed facility, we should review the applicability of conclusions and recommendations in this report. We recommend a general review of final design and specifications by our office to verify that foundation recommendations are properly interpreted and implemented in the design specifications.

We are pleased to be of assistance to you on this phase of the project. When we may be of further service to you or should you have any questions, please contact us.

MINIMA

No 53265

STATE OF

1480111

Very truly yours,

ARDAMAN & ASSOCIATES, INC.

Certificate of Authorization No. 5950

Dustin M. Cone

Assistance Project Engineer

DMC/JPM/dk

Jason P. Manning, P

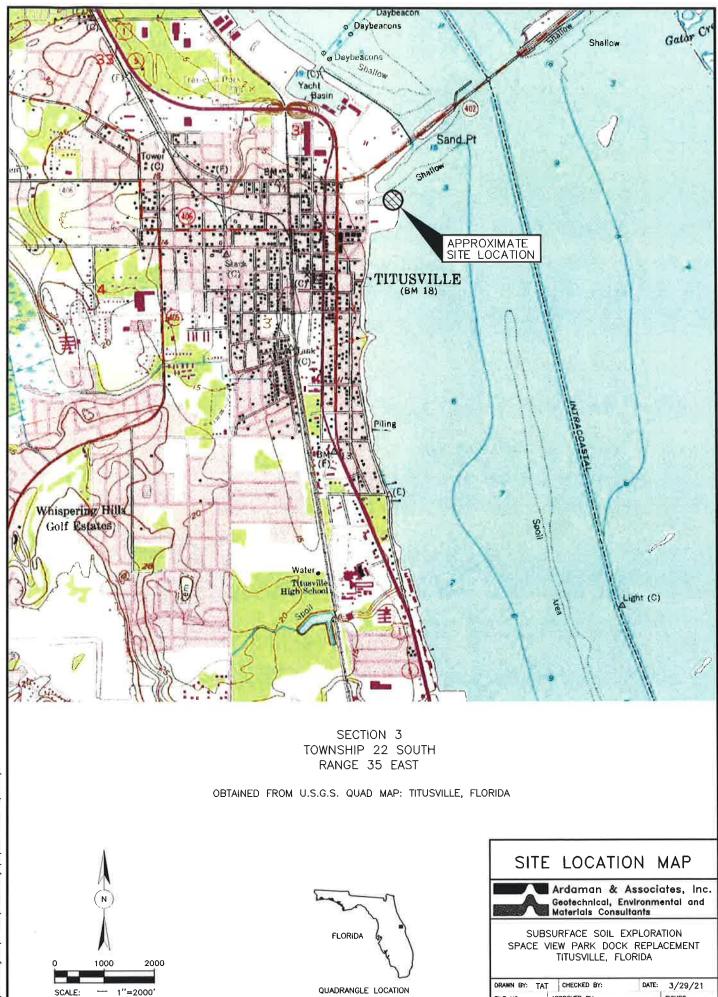
Branch Manager Florida License No. 53265

Jason P.

Manning , P.E.

Digitally signed by: Jason P. Manning, P.E.
DN: CN = Jason P. Manning, P.E. email = Jmanning@ardaman.
com C = US OU = Ardaman and Associates

Date: 2021.04.07 14:49:30 -05:00

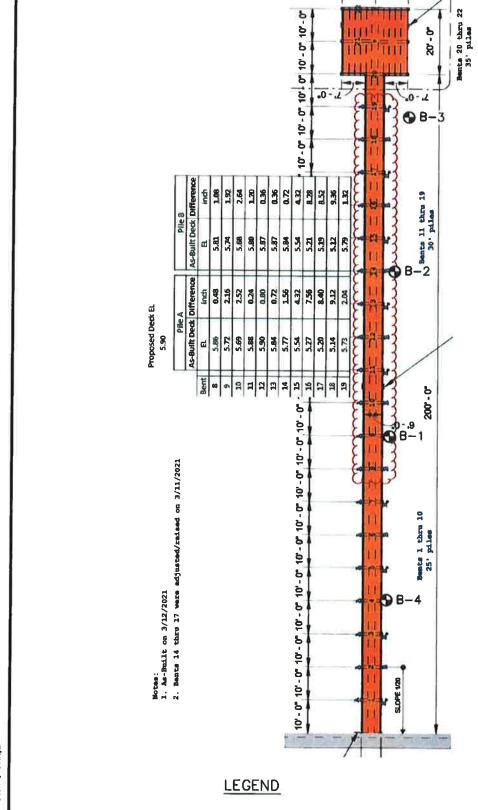


FILE NO. 21-5223

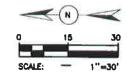
APPROVED BY:

FIGURE:

C:\CAD working files\21-5223\21522301.dwg 3/30/2021 7:31:55 PM, Tom.Toylor



STANDARD PENETRATION TEST (SPT) BORING LOCATION



BORING LOCATION PLAN



Ardaman & Associates, Inc. Geotechnical, Environmental and Materials Consultants

SUBSURFACE SOIL EXPLORATION SPACE VIEW PARK DOCK REPLACEMENT TITUSVILLE, FLORIDA

DRAWN BY: TAT	CHECKED SV:	OATE:	3/29	/21
FILE NO. 21-5223	APPROVED BY:		FIGURE	2

APPENDIX I

Standard Penetration Test Boring Procedures

STANDARD PENETRATION TEST

The standard penetration test is a widely accepted test method of *in situ* testing of foundation soils (ASTM D 1586). A 2-foot long, 2-inch O.D. split-barrel sampler attached to the end of a string of drilling rods is driven 18 inches into the ground by successive blows of a 140-pound hammer freely dropping 30 inches. The number of blows needed for each 6 inches of penetration is recorded. The sum of the blows required for penetration of the second and third 6-inch increments of penetration constitutes the test result or N-value. After the test, the sampler is extracted from the ground and opened to allow visual examination and classification of the retained soil sample. The N-value has been empirically correlated with various soil properties allowing a conservative estimate of the behavior of soils under load.

The tests are usually performed at 5-foot intervals. However, more frequent or continuous testing is done by our firm through depths where a more accurate definition of the soils is required. The test holes are advanced to the test elevations by rotary drilling with a cutting bit, using circulating fluid to remove the cuttings and hold the fine grains in suspension. The circulating fluid, which is a bentonitic drilling mud, is also used to keep the hole open below the water table by maintaining an excess hydrostatic pressure inside the hole. In some soil deposits, particularly highly pervious ones, NX-size flush-coupled casing must be driven to just above the testing depth to keep the hole open and/or prevent the loss of circulating fluid.

Representative split-spoon samples from the soils at every 5 feet of drilled depth and from every different stratum are brought to our laboratory in air-tight jars for further evaluation and testing, if necessary. Samples not used in testing are stored for 30 days prior to being discarded. After completion of a test boring, the hole is kept open until a steady state groundwater level is recorded. The hole is then sealed, if necessary, and backfilled.

APPENDIX II

Soil Boring Profiles

LEGEND

SOIL DESCRIPTIONS

COLORS

- A DARK GRAY OR DARK BROWN (1) ORGANIC MUCK AND SILT (OH,ML) 2) FINE SAND (SP) (B) LIGHT GRAY TO GRAY (3) FINE SAND WITH SILT (SP-SM) (4) SILTY FINE SAND (SM) (5) CLAYEY FINE SAND (SC) (6) CLAY (CL,CH)
 - STANDARD PENETRATION TEST (SPT) BORING
 - N STANDARD PENETRATION RESISTANCE IN BLOWS PER FOOT
- WOH SAMPLER ADVANCED BY STATIC WEIGHT OF HAMMER AND RODS ONLY
- WOR SAMPLER ADVANCED BY STATIC WEIGHT OF RODS ONLY
- SP,SP-SM SM,SC,CH

UNIFIED SOIL CLASSIFICATION SYSTEM

ENGINEERING CLASSIFICATION

I COHESIONLESS SOILS

BLOW COUNT "N"					
0 TO 4 4 TO 10 10 TO 30 30 TO 50 >50					

II COHESIVE SOILS

DESCRIPTION	UNCONFINED COMPRESSIVE STRENGTH, QU, TSF	BLOW COUNT "N"
VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF	1/4 TO 1/2 1/2 TO 1 1 TO 2 2 TO 4	0 TO 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30
HARD	>4	>30

WHILE THE BORINGS ARE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT THEIR RESPECTIVE LOCATIONS AND FOR THEIR RESPECTIVE VERTICAL REACHES, LOCAL VARIATIONS CHARACTERISTIC OF THE SUBSURFACE MATERIALS OF THE REGION ARE ANTICIPATED AND MAY BE ENCOUNTERED. THE BORING LOGS AND RELATED INFORMATION ARE BASED ON THE DRILLER'S LOGS AND VISUAL EXAMINATION OF SELECTED SAMPLES IN THE LABORATORY. THE DELINEATION BETWEEN SOIL TYPES SHOWN ON THE LOGS IS APPROXIMATE AND THE DESCRIPTION REPRESENTS OUR INTERPRETATION OF SUBSURFACE CONDITIONS AT THE DESIGNATED BORING LOCATIONS ON THE PARTICULAR DATE DRILLED.

GROUNDWATER ELEVATIONS SHOWN ON THE BORING LOGS REPRESENT GROUNDWATER SURFACES ENCOUNTERED ON THE DATES SHOWN. FLUCTUATIONS IN WATER TABLE LEVELS SHOULD BE ANTICIPATED THROUGHOUT THE YEAR.

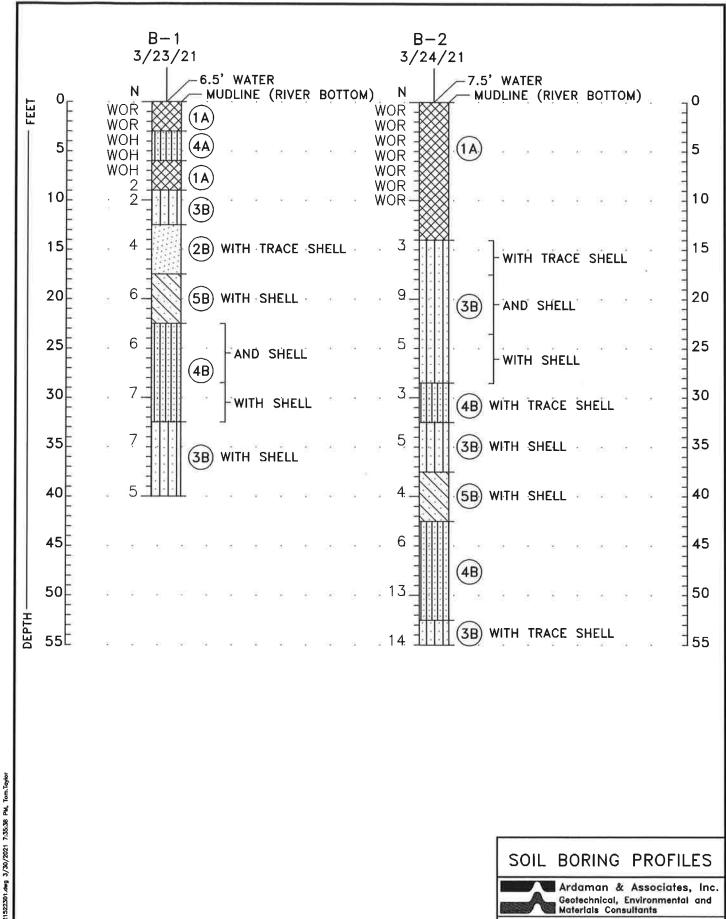
SOIL PROFILES LEGEND



📰 Ardaman & Associates, Inc. Geotechnical, Environmental and Materials Consultants

SUBSURFACE SOIL EXPLORATION SPACE VIEW PARK DOCK REPLACEMENT TITUSVILLE, FLORIDA

DRAWN BY: TAT	CHECKED BY:	DATE:	3/29/21		
FILE NO. 21-5223	APPROVED BY:		FIGURE:		



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SUBSURFACE SOIL EXPLORATION SPACE VIEW PARK DOCK REPLACEMENT TITUSVILLE, FLORIDA

 DRAWN BY:
 TAT
 CHECKED BY:
 DATE:
 3/29/21

 FILE NO.
 21-5223
 APPROVED BY:
 FIGURE:



Ardaman & Associates, Inc. Geotechnical, Environmental and Materials Consultants

SUBSURFACE SOIL EXPLORATION SPACE VIEW PARK DOCK REPLACEMENT TITUSVILLE, FLORIDA

DRAWN BY: TAT CHECKED BY: FILE NO. 21-5223

APPROVED BY:

DATE: 3/29/21 FIGURE:



Ardaman & Associates, Inc.

Geotechnical, Environmental and Materials Consultants

Florida: Bartow, Cocoa, Fort Myers, Miami, Orlando, Port St. Lucie, Sarasota, Tallahassee, Tampa, W. Palm Beach Louisiana: Baton Rouge, Monroe, New Orleans, Shreveport

> 1300 NORTH COCOA BOULEVARD COCOA, FLORIDA 32922 PHONE: 321.632.2503 FAX: 321.636.4657

Job: **2025** RUSH Construction

Cost Code: 01-90-13 Date: **04/19/21 DATE:** April 12, 2021 Amount: 19,594.75 Initials: mel

04/14/2021 9:21:24 AM

C41860 INVOICE Rush Construction, Inc.

6285 Riverfront Center Boulevard

113-21-23-5223A FILE NO.

Titusville, Florida 32780 SERVICE **Engineering**

OFFICE COCOA Attention: Mr. Mark Lucas

accountspayable@rushinc.com

TERMS: Net 30 Days from date of invoice.

Professional Services Relative to		
Subsurface Soil Exploration and Geotechnical Engineering Evaluation		
Dock Repairs Project – Brevard County Parks & Recreation		
Space View Park		
Titusville, Florida		
Original Project Budget:	\$ 18,416.00	
Additional Costs for Extending Two Borings Deeper:		
Additional Boring Footage (50-100'): 35 linear feet @ \$ 29.25/ft.	\$ 1,023.75	
Project Engineer: 1 hour @ \$ 120.00/hr.	\$ 120.00	
CADD Draftsman: 0.5 hour @ \$ 70.00/hr.	\$ 35.00	
TOTAL INVOICE AMOUNT		<u>\$ 19,594.75</u>
Please send your payment to:		
ARDAMAN & ASSOCIATES INC PO BOX 911668		
Denver CO 80291-1668		

Please write the invoice number on your check. Thank you!



Ardaman & Associates, Inc.

Geotechnical, Environmental and Materials Consultants

April 13, 2021 File No. 21-23-5223

Rush Construction 6285 Riverfront Center Boulevard Titusville, Florida 32780

Attention:

Mr. Mark Lucas

Subject:

Lateral Pile Capacity Analysis Costs

Brevard County Parks & Recreation Dock Replacement at

Space View Park Titusville, Florida

Dear Mr. Lucas:

As discussed during the project team teleconference on April 8, 2021, Ardaman & Associates, Inc. will perform one iteration of lateral load capacity analysis for the final pile configuration selected by the design team. For this analysis, we will need to be provided with the following information:

- total length of the new piles;
- embedment depth below the mud line of the new piles;
- diameter of the new piles;
- location and magnitude of the lateral load to be analyzed for; and
- whether the new piles will be in a "fixed head" or "free head" condition.

The additional cost of the lateral load analysis will not exceed \$1,500.00 without prior approval from the client. Please provide written approval for these additional costs at your earliest convenience. Feel free to contact us if you have any questions or require additional information.

Very truly yours.

ARDAMAN & ASSOCIATES, INC.

Jason P. Manning, P.E.

Branch Manager

Approved Rush Construction Mexicon Make hear Por.

Mark Lucas

From:

Butcher, Michael <michael.butcher@brevardfl.gov>

Sent:

Wednesday, April 28, 2021 4:56 PM

To:

Mark Lucas; David Tom; Josh Adams

Cc:

Matt Tate; Tony Landry; Mike Lapinski; com-inbound-brevard-county-spaceview-

park@procoretech.com

Subject:

RE: Space View Lateral Load Analysis

Mark,

per our discussion and with the below calculations please place the order for the 50' pilings from bend 8-19 (26) and the 55" pilings for the observation deck platform. (10). Thank you to all involved.

Michael Butcher

Construction Manager

Brevard County Parks Operations

2725 Judge Fran Jamieson Way Building B Suite 203 P: (321) 633-2046 Cell: (321) 615-8214

Fax(321633-2198

http://www.BrevardParks.com





The comments and opinions expressed herein are those of the author of this message and may not reflect the policies of the Brevard County Board of Commissioners

From: Mark Lucas <mlucas@rushinc.com> Sent: Wednesday, April 28, 2021 4:52 PM

To: Butcher, Michael <michael.butcher@brevardfl.gov>; David Tom <dtom@cegengineering.com>; Josh Adams

<jadams@cegengineering.com>

Cc: Matt Tate <mtate@rushinc.com>; Tony Landry <tlandry@rushinc.com>; Mike Lapinski <mlapinski@rushinc.com>;

com-inbound-brevard-county-spaceview-park@procoretech.com

Subject: FW: Space View Lateral Load Analysis

[EXTERNAL EMAIL] DO NOT CLICK links or attachments unless you recognize the sender and know the content is safe.

Michael/David/Josh,

Hot off the press! Results of the lateral analysis from Ardaman & Associates. For your information and use.

Thank you,

Mark Lucas
Project Manager

RUSH CONSTRUCTION INC.

Performance with Integrity Since 1984

321.267.8100 phone 321.302-2107 mobile 321.267.9944 fax

www.rushinc.com

From: Manning, Jason < JManning@ardaman.com>

Sent: Wednesday, April 28, 2021 4:48 PM To: Mark Lucas <mlucas@rushinc.com>

Cc: Matt Tate <mtate@rushinc.com>; Tony Landry <tlandry@rushinc.com>; com-inbound-brevard-county-spaceview-

park@procoretech.com

Subject: RE: Space View Lateral Load Analysis

Mark,

We have completed the lateral analysis of the 50-foot and 55-foot long piles. Here is a summary of the findings so far:

- 1. For the 50-foot piles, the 12-inch top of pile diameter is okay as long as the minimum bottom of pile diameter is 8 inches.
- 2. For the 55-foot piles, the 12-inch top of pile diameter is okay as long as the minimum bottom of pile diameter is 9 inches.
- 3. For both pile lengths we are calculating approximately 25 inches of deflection of the pile at the mudline due to the 750 pound lateral load acting just below the girder level for a free head condition.
- 4. To address the excessive deflection in the free head condition, we recommend that adequate bracing be incorporated in to the dock design to create a fixed head condition.
- 5. For both pile lengths we are calculating approximately 1.2 to 1.5 inches of deflection of the pile at the mudline due to the 750 pound lateral load acting just below the girder level for a fixed head condition.

Please provide this information to the structural engineer for consideration. We will have the written report ready within the next few days.

Jason P. Manning, P.E. **Branch Manager**

Ardaman & Associates, Inc.

1300 N. Cocoa Boulevard Cocoa, Florida 32922 Ph. (321) 632-2503 Fax. (321) 636-4657 imanning@ardaman.com

From: Mark Lucas [mailto:mlucas@rushinc.com] Sent: Wednesday, April 28, 2021 12:58 PM To: Manning, Jason < JManning@ardaman.com>

Cc: Matt Tate <mtate@rushinc.com>; Tony Landry <tlandry@rushinc.com>; com-inbound-brevard-county-spaceview-

park@procoretech.com

Subject: Space View Lateral Load Analysis

A CAUTION: This email originated from an external sender. Verify the source before opening links or attachments.



Jason,

Any word on the report today?

Thank you,

Mark Lucas
Project Manager
RUSH CONSTRUCTION INC.
Performance with Integrity Since 1984
321.267.8100 phone
321.302-2107 mobile
321.267.9944 fax
www.rushinc.com



May 4, 2021 File No. 21-23-5223A

Rush Construction 6285 Riverfront Center Boulevard Titusville, Florida 32780

Attention:

Mr. Mark Lucas

Subject:

Pile Foundation Lateral Load Analysis

Dock Repairs Project Space View Park Titusville, Florida

Dear Mr. Lucas:

Ardaman & Associates, Inc. (A&A) recently performed a subsurface soil exploration and geotechnical engineering evaluation for the above referenced project. Recommendations for the installation of 12-inch butt diameter timber pile foundations to support the new dock structure as well as axial pile capacities were presented in our April 7, 2021, Subsurface Soil Exploration report (A&A File No. 21-23-5223A).

Pile foundation design parameters were provided by Rush Construction, Inc. and by Mr. Thomas Adams, P.E. with Adams Engineering, Inc., the project Structural Engineer. The design parameters provided to us and utilized in our analysis are as follows:

- The piles to be used are 12-inch butt diameter (top diameter) pressure treated Southern Yellow Pine timber piles;
- The piles will be 50 feet (dock piles) and 55 feet (gazebo piles) in length with 1-foot of cutoff at the surface;
- Dock piles: top of pile El. +9.40 and pile tip El. -39.60;
- Gazebo piles: top of pile El. +13.90 and pile tip El. -40.10;
- River mudline El. -8.00;
- Design lateral load is 0.75 kips acting approximately 12 inches below dock girder level;

Lateral capacity and deflection of the piles were calculated utilizing the computer software program "L-Pile V2013.7.04". As requested, we analyzed the piles in a "free head" condition. Based on the results of our analyses, we estimate that the 12-inch timber dock piles and gazebo piles installed to approximately 32 feet below the existing mudline within the river would experience approximately 25 inches of deflection at the mudline in the "free head" condition.

We also analyzed the piles in a "fixed head" condition. Based on this additional analysis, we estimate that the 12-inch timber dock piles and gazebo piles installed to approximately 32 feet below the existing mudline within the river would experience approximately 1½ to 1½ inches of

deflection at the mudline. To address the excessive deflection in the "free head" condition, we recommend that adequate bracing be incorporated into the dock design to create a "fixed head" condition.

The results of our analyses also indicate that the 50-foot long dock piles should have a minimum tip diameter (bottom diameter) of 8 inches and that the 55-foot long gazebo piles should have a minimum tip diameter of 9 inches.

We note that the analyses presented in this report are for individual piles and do not account for the reduced efficiency of pile groups. Pile spacing should be at least 3.0 pile diameters center to center to prevent reduced lateral capacities caused by the pile to pile interaction. If the pile spacing is closer than 3.0 pile diameters, we should be consulted to provide a lateral capacity reduction factor for the proposed piles.

Should the embedment depth, pile types or any other assumptions from above be different than planned, we must be given the opportunity to review their requirements, since they may impact our recommendations.

The analyses and recommendations submitted herein are based on the data obtained from the soil borings presented in our previous geotechnical report. This report does not reflect any variations which may occur adjacent to or between the borings. The nature and extent of the variations between the borings may not become evident until during construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations presented in this report after performing on-site observations during the construction period and noting the characteristics of the variations. This study does not include an evaluation of the environmental (ecological or hazardous/toxic material related) condition of the site and subsurface.

This report has been prepared for the exclusive use of Rush Construction, Inc. in accordance with generally accepted geotechnical engineering practices. In the event any changes occur in the design, nature, or location of the proposed facility, we should review the applicability of conclusions and recommendations in this report. We recommend a general review of final design and specifications by our office to verify that foundation recommendations are properly interpreted and implemented in the design specifications.

We are pleased to be of assistance to you on this phase of the project. When we may be of further service to you or should you have any questions, please contact us.

No 53265

Very truly yours,

ARDAMAN & ASSOCIATES, INC.

Certificate of Authorization No. 5950

Kevin Ferguson, P.E. Senior Project Engineer

Florida License No. 60712

Jason P. Manning,

Branch Manager

Florida License No. 53265

Jason P.

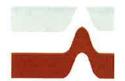
Manning , P.E.

Digitally signed by: Jason P. Manning, P.E.

DN: CN = Jason P. Manning, P.E. email = jmanning@ardaman.

com C = US OU = Ardaman and Associates

Date: 2021.05.04 15:25:28 -05'00'



Ardaman & Associates, Inc.

Geotechnical, Environmental and Materials Consultants

Florida: Bartow, Cocoa, Fort Myers, Miami, Orlando, Port St, Lucie, Sarasota, Tallahassee, Tampa, W. Palm Beach Louisiana: Baton Rouge, Monroe, New Orleans, Shreveport

1300 NORTH COCOA BOULEVARD COCOA, FLORIDA 32922

PHONE: 321.632.2503 FAX: 321.636.4657

DATE: May 7, 2021

INVOICE C41888

Rush Construction, Inc.

Mr. Mark Lucas

6285 Riverfront Center Boulevard

Titusville, Florida 32780

FILE NO.

113-21-23-5223A

SERVICE

Engineering

OFFICE

COCOA

accountspayable@rushinc.com

TERMS: Net 30 Days from date of invoice.

Attention:

Professional Services Relative to Lateral Load Analyses Dock Repairs Project - Brevard County Parks & Recreation Space View Park Titusville, Florida **TOTAL INVOICE AMOUNT** \$ 1,500.00 Please send your payment to:

ARDAMAN & ASSOCIATES INC PO BOX 911668 Denver CO 80291-1668

Please write the invoice number on your check. Thank you!



May 6, 2021

Mr. Mark Lucas RUSH Construction, Inc. 6285 Riverfront Center Blvd. Titusville, FL 32780

Reference: Space View Park Improvements

Subject: Price Proposal for Longer Timber Piles - UPDATED

Mr. Lucas,

RUSH Marine, LLC is pleased to submit this proposal for the above referenced project to provide all supervision, labor, materials, and equipment required to perform the following scope of work at the South Site:

Scope of Work to Include:

Item 1 - Long Pier

- 1. Replace 26 EA timber piles with new 50' timber piles (8" minimum diameter) with pile wrap at Bents 7 thru 19.
- 2. Remove Roof Structure.
- 3. Replace 10 EA timber piles with new 55' timber piles (9" minimum diameter) with pile wrap for Roof Structure.
- 4. Reinstall Roof Structure.
- 5. New Stainless Steel Hardware will be required for the larger diameter piles.

Price for Item 1: \$75,776.00

Seventy-Five Thousand Seven Hundred Seventy-Six Dollars



Item 2 – Standby Equipment

Cost of Equipment only to be on standby based on the following timeline:

- Settlement issue identified and RFI sent March 12, 2021
- Start Standby on March 15, 2021
- Submit Proposal on April 14, 2021
- Receive Notice to Proceed on April 28, 2021
- Longer piles deliver to site on May 18, 2021
- Standby from March 15, 2021 thru May 18, 2021 (47 Working Days)

Cost for Item 2: \$62,863.00

Sixty-Two Thousand Eight Hundred Sixty-Three Dollars

Item 3 - Additional Cross Bracing for Long Pier

Install treated 2x6 cross bracing at additional 9 bents of South Long Pier using stainless steel hardware, in accordance with drawings.

Price for Item 3: \$10,085.00

Ten Thousand Eighty-Five Dollars

The total cost for these three items above is \$148,724.00 (One Hundred Forty-Eight Thousand Seven Hundred Twenty-Four Dollars).

Excluded Items:

- Design and Engineering is excluded.
- Pile Load Testing is excluded.
- Any additional work at North Short Pier is excluded.
- Any additional Cross Bracing at North Short Pier is excluded. RUSH Marine reserves the right to request for additional cost if cross bracing is added or modified.



Clarifications:

- 1. All work shall be performed between 7:00 am 5:30 pm, Monday through Friday, excluding holidays, weekends, and special events.
- 2. RUSH Marine shall have unimpeded access to the work area from land and water during the time periods noted above.
- 3. This proposal is valid for 30 days from the date received.
- 4. RUSH Marine reserves the right to request additional cost if additional work is required related to cross bracing.

Please do not hesitate to contact us, should you have any questions, regarding this proposal. We would like to thank you for this opportunity, and look forward to working with your team.

Sincerely, **RUSH MARINE, LLC**

Matt Tate
Estimator / Sr. Project Manager

Long Pier

1.4-0-	<u> </u>		_						_		-	
LABOR	Durartion			g Rate		Duration		MOM	_	T RATE		dened Labor
Superintendent	80	HR	\$	56.00	_	0		HR	\$	84.00	\$	6,361,60
Foreman	80	HR	\$	45,00	-	0	-	HR	\$	67.50	\$	5,112.00
Operator	80	HR	\$	38.00	_	20		HR	\$	57.00	\$	5,935,60
Pile Driver/Carpenter	80	HR	\$	25.00		20		HR	\$	37,50	\$	3,905.00
Pile Driver/Carpenter	80	HR	\$	25.00	_	20		HR	\$	37.50	\$	3,905.00
		HR			_			HR	\$	-	\$	
		HR						HR	\$		\$	
	320		7		,	60				Total	\$	25,219
Equipment	Durartion	UOM							Mo	onth RATE		Total
Barge	10	Days							\$	13,000.00	\$	6,955.00
Crane	10	Days							\$	8,500.00	\$	4,547.50
Barge Thrusters	10	Days							\$	3,500.00	\$	1,872.50
Forklift	10	Days							\$	1,275.00	\$	682,13
Pile Hammer	10	Days							\$	2,000,00	\$	1,070.00
											\$	\.
											\$	
							-			Total	\$	15,127
Subcontractor	Qty	UOM								RATE		Total
						"					\$	-
											\$	12
											\$	-
					-					Total	\$	
Materials	Qty	UOM				RATE		Subtotal		Taxes		Total
50' Treated Timber Piles, 8" minimum	26	EA			\$	550.00	\$	14,300.00	\$	1,001.00	\$	15,301.00
55' Treated Timber Piles, 9" minimum	10	EA			\$	816.00		8,160.00	\$	571.20	\$	8,731,20
Pile Wrap, 5'x300' Roll	1	EA			\$	458.76	\$	458.76	\$	32.11	\$	490,87
SS Rods, 24"	72	EA			\$	11.00	\$	792.00	\$	55.44	\$	847.44
					Ť		\$	-	\$		\$	
							\$	_	\$		\$	34
					1		\$		\$		\$	
Fuel, 50 gallons per day	500	GA			\$	3.50		1,750.00	$\overline{}$	122.50	\$	1,872.50
, doi, oo ganono por day	1 300	UA			ŢΨ	3,30	ΙΨ_	1,750.00	Ψ	Total	\$	27,243
Freight (No Toyon)	OTV	LIOM			I					RATE	_	Total
Freight (No Taxes)	QTY	UOM	_		1				-	150.00	\$	150.00
50' Piles	1	EA	_		+-		_		\$			
55' Piles	1	EA	-		╁		-		\$	150.00	\$	150.00
		EA			-				150		\$	
						DTOTAL				Total	\$	300 67 990
				DI IOI		BTOTAL				4.040/	- 3	67,889
						rine Insu				1.24%	\$	842
RUSH Marine Overhead										5.00%	\$	3,437
				RU		Marine P	rofit			5.00%	\$	3,608
						TOTAL					\$	75,776

Standby

				Equipm	ent Standby						
LABOR	Durartion	UOM	R	eg Rate	Duration	UO	М	C	T RATE	Bure	dened Labor
Superintendent		HR	\$	56.00	0	Н	₹	\$	84.00	\$	
Foreman	0	HR	\$	45.00	0	H	₹	\$	67.50	\$	
Operator	0	HR	\$	38.00		H	₹	\$	57.00	\$	
Pile Driver/Carpenter	0	HR	\$	25.00	0	H	₹	\$	37.50	\$	
Pile Driver/Carpenter	0	HR	\$	25.00	0	H	₹	\$	37.50	\$	
		HR				Hf	₹	\$		\$	
		HR			0	H	₹	\$	<u>.</u>	\$	
	0				0				Total	\$	
Equipment	Durartion	UOM						Mc	nth RATE		Total
Barge	47	Days						\$	13,000.00	\$	32,688.50
Crane	47	Days						\$	8,500.00	\$	21,373.25
Barge Thrusters	47	Days						\$	3,500.00	\$	8,800.75
										\$	3
										\$	2
										\$	
										\$	
									Total	\$	62,863
Subcontractor	Qty	UOM							RATE		Total
										\$	¥
										\$	
										\$	
									Total	\$	
Materials	Qty	UOM			RATE	Subt	otal		Taxes		Total
						\$	•	\$	- 2	\$	
						\$	340	\$. 2	\$	
						\$	(4)	\$		\$	
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						\$:::	\$		\$	
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						\$		\$	(a)	\$	
						\$		\$	-	\$	
									Total	\$	W 5.
Freight (No Taxes)	QTY	UOM							RATE		Total
		EA								\$	
		EA								\$	3
		EA								\$	
									Total	\$	
					SUBTOTAL					\$	62,863
				RUSH	Marine Inst	urance			0.00%	\$	8 ¥ €
				RUSH	Marine Ov	erhead			0.00%	\$	-
				RUS	SH Marine F	Profit			0.00%	\$	
					TOTAL					\$	62,863

Cross Bracing

·	lr lr	nstall	Additional	Cros	s Bracin	g - Lo	ong Pier				
LABOR	Durartion	UOM	Reg Rate		uration	ι	ОМ	C	OT RATE	Bure	dened Labor
Superintendent	16	HR	\$ 56.00		0		HR	\$	84.00	\$	1,272.32
Foreman	16	HR	\$ 45.00		0		HR	\$	67.50	\$	1,022.40
Operator	16	HR	\$ 38.00		4		HR	\$	57.00	\$	1,187.12
Pile Driver/Carpenter	16	HR	\$ 25.00		4		HR	\$	37.50	\$	781.00
Pile Driver/Carpenter	16	HR	\$ 25.00		4		HR	\$	37.50	\$	781.00
		HR					HR	\$		\$	*
		HR					HR	\$		\$	
	64				12				Total	\$	5,044
Equipment	Durartion	UOM						Mo	onth RATE		Total
Barge	2	Days						\$	13,000.00	\$	1,391.00
Crane	2	Days						\$	8,500.00	\$	909.50
Barge Thrusters	2	Days						\$	3,500.00	\$	374.50
Forklift	2	Days						\$	1,275.00	\$	136,43
Pile Hammer	0	Days						\$	2,000.00	\$	
										\$	
										\$	-
									Total	\$	2,811
Subcontractor	Qty	UOM							RATE		Total
										\$	
										\$:*:
										\$	-
			N-						Total	\$	
Materials	Qty	UOM			RATE	Su	ibtotal		Taxes		Total
Treated 2x6x16'	9	EΑ		\$	24.71	\$	222.39	\$	15.57	\$	237.96
5/8" SS Bolts, 18 in	36	EA		\$	9.18	\$	330.48	\$	23.13	\$	353.61
3/4" SS Bolts, 18 in	9	EA		\$	10.24	\$	92,16	\$	6.45	\$	98.61
5/8" Nuts and Washers	72	EA		\$	0.58	\$	41.76	\$	2.92	\$	44.68
3/4" Nuts and Washers	18	EA		\$	3.73	\$	67.14	\$	4.70	\$	71.84
						\$	÷	\$	<u> </u>	\$	<u> </u>
						\$		\$	i i	\$	
Fuel, 50 gallons per day	100	GA		\$	3.50	\$	350.00	\$	24.50	\$	374.50
									Total	\$	1,181
Freight (No Taxes)	QTY	UOM							RATE		Total
		EA								\$	37
		EA								\$	¥
		EΑ								\$:=:
					_3)			9,	Total	\$	
				SUE	BTOTAL					\$	9,036
			RUSH	l Ma	rine Insu	rance	•		1,24%	\$	112
			RUS	Н Ма	rine Ove	rhead	k		5.00%	\$	457
			RU	ISH N	Marine Pi	rofit			5.00%	\$	480
						• • • • • • • • • • • • • • • • • • • •					

SPL - Rockledge 2135 South US 1 Rockledge, FL 32955 321-632-5566 F'ax: 321-632-5171

* NO1TATOUQ *

Sales Invoice:

Ordered by: Matt/Verbal Order Number: 16340000 P.O. Number: Quote Only

Bill To: 000643 RUSH MARINE, LLC

6285 Riverfront Center Blvd FL 32780 Titusville

Invoice Date: 05/03/21 03:32 PM Terms: NET 15 DAYS Due Date: 05/18/21

Ship VIA:

Ship To: Quote Only

SHIP BKO DESCRIPTION	BRD FT	UNIT PRICE	EXTENSION
26 Poles	0	550.00	14,300.00
8" Top 50ft 2.5 MP 10 Poles	0	816.00	8,160.00
9" Top 55ft 2.5 MP 1 Delivery Charge 2 Direct Truck From Mill	0	300.00	300.00

ALL SALES FINAL, NO RETURNS	Subtotal	22,760.00
\$50.00 Fee will apply on Returned Checks	Sales Tax	1,347.60
Above prices are discounted for cash or check	Surtax	50.00
Picked by: Driver: Date: Total Brd/Ft: 0 Total Weight:	 Total	24,157.60



Invoice Address RUSH Marine, LLC 6285 Riverfront Center Blvd. Titusville, FL, 32780

Orlando

5790 Beggs Rd. Lockhart, FL

Quotation

Quote No Quote Date 256510 05/05/2021

Delivery Address 10 East Broad Street Titusville, FL, 32796 Customer Your Ref RUSH2

Taken By Sales Rep 8T & 9T Piles John Miller Default

Contact Name

Mike Tate

Contact Number Contact Fax 321-474-2560 321-267-9944



Page 1 of 1

Special Instructions Notes

				[
_ine	Product Code	Description	Qty/Footage	Price	Per	Total
	zz_ \$SOPiling2.5CCA_ 0247	8" Top x 50' Piling 2.5 CCA	34 Each	586.67	Each	19,946.78
2	BOOM Charge	Boom Charge - three trucks @ \$150 per truck				450.00
3	Freight	Freight - job site delivery at \$100 per truck load at 3 truckloads				300.00
	zz_ \$SOPiling2,5CCA_ 0248	9" Top x 55' Piling 2.5 CCA	10 Each	906.67	Each	9,066.70

This quote is good for _____ days

Total Amount	\$29,763.48
Sales Tax	\$1,790.81
Quotation Total	\$31,554.29

By your signature below, you are agreeing to the Terms and Conditions set forth on back or attached.

Buyer

Date



Space View Park Improvements

