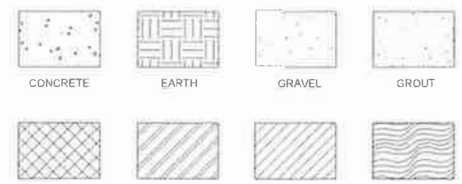
STRUCTURAL ABBREVIATIONS

THE FOLLOWING ABBREVIATIONS MAY BE USED IN THE STRUCTURAL DRAWINGS

@	AT	L	LENGTH
&	AND	LBS	POUNDS
#	NUMBER	Ld	DEVELOPMENT LENGTH
AHU	AIR HANDLING UNIT	LLBB	LONG LEGS BACK TO BACK
ALT	ALTERNATE	LLH	LONG LEG HORIZONTAL
ARCH	ARCHITECT	LLV	LONG LEG VERTICAL
BLDG	BUILDING	LONG	LONGITUDINAL
BM	BEAM	M	MOMENT
B/	BOTTOM OF	MAS	MASONRY
BPL	BENT PLATE	MAX	MAXIMUM
BRG	BEARING	MC	MOMENT CONNECTION
BTWN	BETWEEN	MECH	MECHANICAL
C	CAMBER / COMPRESSION	MEZZ	MEZZANINE
CANT	CANTILEVER	MFR	MANUFACTURER
CIP	CAST IN PLACE	MIN	MINIMUM
CJ	CONSTRUCTION JOINT	MISC	MISCELLANEOUS
CL	CENTER LINE	NIC	NOT IN CONTRACT
CLR	CLEAR	NS	NEAR SIDE
CMU	CONCRETE MASONRY UNIT	NTS	NOT TO SCALE
COL	COLUMN	O.C.	ON CENTER
CONC	CONCRETE	OD	OUTSIDE DIAMETER
CONN	CONNECTION	OPH	OPPOSITE HAND
CONT	CONTINUOUS	OPNG	OPENING
CVR	COVER	OPP	OPPOSITE
D	DEPTH	OVS	OVERSIZED
db	BAR DIAMETER	P	AXIAL LOAD
DIA	DIAMETER	PAF	POWDER ACTUATED FASTENER
DWG(S)	DRAWING(S)	PCF	POUNDS PER CUBIC FOOT
DWL	DOWEL	PCY	POUNDS PER CUBIC YARD
E	ECCENTRICITY	PERP	PERPENDICULAR
EA	EACH	PL	PLATE
EF	EACH FACE	PLF	POUNDS PER LINEAL FOOT
EJ	EXPANSION JOINT	PSF	POUNDS PER SQUARE FOOT
EL	ELEVATION	PSI	POUNDS PER SQUARE INCH
EMBED	EMBEDMENT	PT	POST TENSIONED
ENG	ENGINEER	QTY	QUANTITY
EQ	EQUAL	R	REACTION
EQUIP	EQUIPMENT	RAD	RADIUS
EQUIV	EQUIVALENT	REF	REFERENCE
ES	EACH SIDE	REINF	REINFORCING
EW	EACH WAY	REQD	REQUIRED
EXIST	EXISTING	REV	REVISION
EXP	EXPANSION	RTU	ROOF TOP UNIT
EXT	EXTERIOR	SC	SLIP CRITICAL
FAB	FABRICATE	SCHED	SCHEDULE
f'c	CONCRETE STRENTH	SDS	SELF DRILLING SCREW
f'm	MASONRY STRENGTH	SECT	SECTION
FD	FLOOR DRAIN	SIM	SIMILAR
FDN	FOUNDATION	SLBB	SHORT LEGS BACK TO BACK
FF	FINISH FLOOR	SOG	SLAB ON GRADE
FLR	FLOOR	SPEC	SPECIFICATION
FS	FAR SIDE	SS	STAINLESS STEEL
FTG	FOOTING	SSL	SHORT SLOTTED
FV	FIELD VERIFY	STD	STANDARD
Fy	YIELD STRENGTH	STIFF	STIFFNER
GALV	GALVANIZED	STL	STEEL
GR	GRADE	SW	SHEAR WALL
H	HORIZONTAL	SYM	SYMMETRICAL
HSS	HOLLOW STRUCTURAL SECTION	T	TENSION
ID	INSIDE DIAMETER	T/	TOP OF
INT	INTERIOR	TRANS	TRANSVERSE
JT	JOINT	TYP	TYPICAL
K	KIPS (1 KIP = 1000 LBS)	UNO	UNLESS NOTED OTHERWISE
KSF	KIPS PER SQUARE FOOT	WP	WORK POINT
KSI	KIPS PER SQUARE INCH	WT	WEIGHT

MATERIAL IDENTIFICATION

THE FOLLOWING MATERIAL IDENTIFICATIONS MAY BE USED IN THE STRUCTURAL DRAWINGS



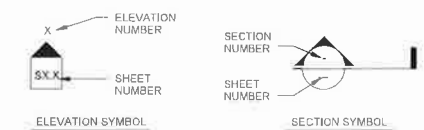
DRAWING INTERPRETATION:

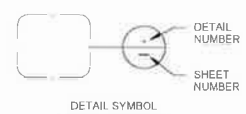
DRAWING VIEWS LABELED AS "TYPICAL"

PARTIAL PLANS, ELEVATIONS, SECTIONS, DETAIL, OR SCHEDULES LABELED WITH "TYPICAL" BEGINNING OF THEIR TITLE APPLY TO SITUATIONS OCCURING ON A PROJECT THAT ARE SAME OR SIMILAR TO THOSE INDICATED WHETHER OR NOT THEY ARE KEYED AT EACH LOCATION. APPLICABILITY CAN BE DETERMINED BY THE TITLE OF EACH VIEW.

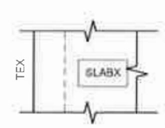
VIEW REFERENCE SYMBOLS

THE FOLLOWING SYMBOLS MAY BE USED TO REFER TO OTHER VIEWS IN THE DRAWINGS





FOUNDATION AND COLUMN DESIGNATIONS



TEX DENOTES THICKENED SLAB EDGE FOOTING TYPE, SEE SCHEDULE

SLABX DENOTES SLAB ON GRADE TYPE SEE SCHEDULE

DESIGN CRITERIA

ALL STRUCTURAL WORK FOR THIS PROJECT HAS BEEN ENGINEERED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, 2014 ASCE-7 10 "MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES.

ALL CONSTRUCTION SHALL COMPLY WITH THE REQUIREMENTS AND SPECIFICATIONS OF THESE CODES AND THEIR REFERENCED STANDARDS, AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL CODES, STANDARDS, REGULATIONS AND LAWS.

THIS STRUCTURE IS LOCATED IN A "WIND BORNE DEBRIS REGION" AS DEFINED BY THE FLORIDA BUILDING CODE AND THE STRUCTURE HAS BEEN DESIGNED AS AN "ENCLOSED" BUILDING. ALL EXTERIOR WALL OPENINGS SHALL HAVE EQUIPMENT OR COVERINGS WHICH MEET THE IMPACT RESISTANT REQUIREMENTS OF FBC 1609 "PROTECTION OF OPENINGS". CURRENT NOA'S (NOTICE OF ACCEPTANCE) CERTIFICATIONS SHALL BE SUBMITTED FOR ALL WINDOWS, DOORS AND COVERINGS.

THE GENERAL CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION SHOWN ON THE DRAWINGS WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS. ANY QUESTIONS OR DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD BEFORE STARTING CONSTRUCTION.

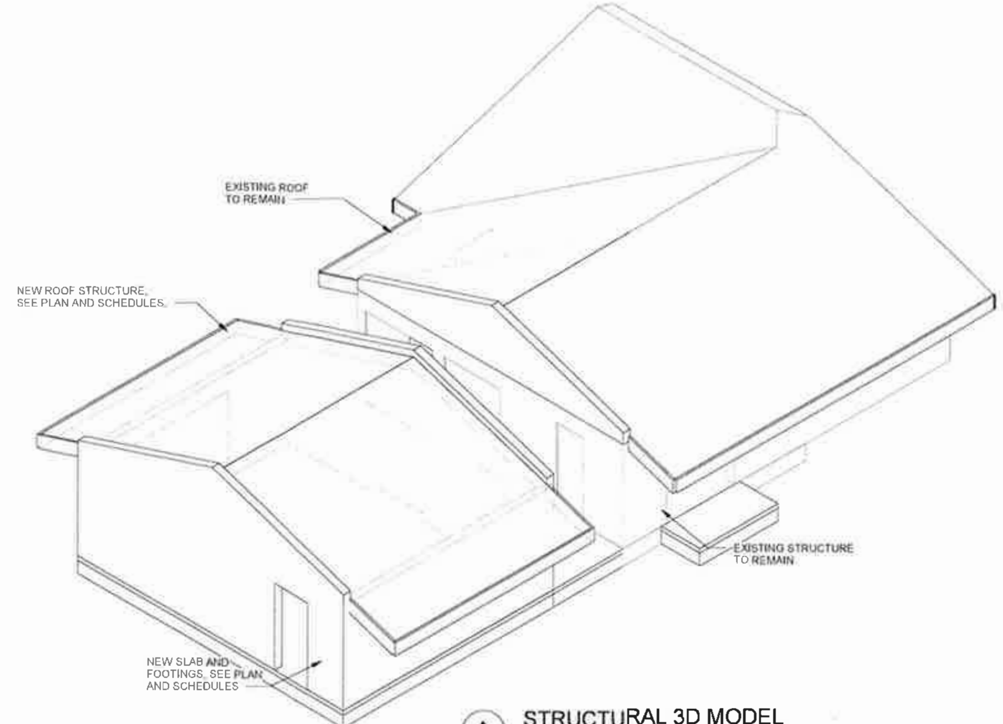
ALL STRUCTURAL WORK SHALL BE INSPECTED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE AND ALL LOCAL ORDINANCES. THE OWNER SHALL ENGAGE AN EXPERIENCED, QUALIFIED INSPECTION AGENCY, SUBJECT TO THE REVIEW BY THE ARCHITECT OR ENGINEER TO PERFORM ALL INSPECTION WORK AS REQUIRED.

THE CONTRACTOR SHALL PROTECT ANY ADJACENT PROPERTY, HIS OWN WORK AND THE PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS AND METHODS, AND JOB SITE SAFETY INCLUDING ALL OSHA REQUIREMENTS.

THE STRUCTURE IS DESIGNED TO BE STRUCTURALLY SOUND WHEN COMPLETED. PRIOR TO COMPLETION, THE CONTRACTOR IS RESPONSIBLE FOR STABILITY AND TEMPORARY BRACING, INCLUDING, BUT NOT LIMITED TO, LOAD BEARING CONCRETE AND MASONRY WALLS, STRUCTURAL STEEL FRAMES, FLOOR AND ROOF FRAMING. WHEREVER THE CONTRACTOR IS UNSURE OF THE REQUIREMENTS, THE CONTRACTOR SHALL RETAIN A FLORIDA LICENSED ENGINEER TO DESIGN AND INSPECT THE TEMPORARY BRACING AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION

DESIGN GRAVITY LOADS	
LOCATION	UNIFORM LIVE LOAD
ROOF	20 PSF

DESIGN WIND LOADS	
WIND SPEED (3 SECOND GUST)	V=150 MPH
RISK CATEGORY	CATEGORY II
EXPOSURE	B
MEAN ROOF HEIGHT	12 FT.
INTERNAL PRESSURE COEFFICIENTS	+/-0.18



A STRUCTURAL 3D MODEL

STRUCTURAL SHEET LIST

Sheet Number	Sheet Name	Sheet Issue Date	REVISIONS: Current Revision	Current Revision Date	Current Revision Description
S0.0	STRUCTURAL COVER SHEET & GENERAL NOTES	07/27/16			
S1.0	PLANS AND SCHEDULES	07/27/16			
S2.0	WALL ELEVATIONS	07/27/16			
S3.0	SECTIONS AND DETAILS	07/27/16			

FOUNDATIONS AND SOIL PREP

FOUNDATIONS HAVE BEEN DESIGNED TO BEAR ON UNDISTRUBED SOILS OR BE PROPERLY COMPACTED FILL HAVING AN ALLOWABLE BEARING CAPACITY OF 2000 PSF.

THE CONTRACTOR SHALL OBSERVE WATER CONDITIONS AT THE SITE AND TAKE THE NECESSARY PRECAUTIONS TO INSURE THAT THE FOUNDATION EXCAVATIONS REMAIN DRY DURING CONSTRUCTION. PROVIDE FOR DEWATERING AS NECESSARY.

THE CONTRACTOR SHALL USE EXTREME CAUTION DURING EXCAVATION. EXCAVATION SHALL BE PERFORMED IN SUCH A MANNER AS TO MAINTAIN THE STRUCTURAL INTEGRITY OF ALL EXISTING STRUCTURES TO REMAIN. PROVIDE TEMPORARY SHORING AS REQUIRED.

AS A MINIMUM, ALL SOILS BELOW THE BUILDING SHALL BE COMPACTED TO WITHIN TWO FEET BELOW BEARING TO 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY.

CAST IN PLACE CONCRETE

ALL REINFORCED CONCRETE WORK SHALL BE IN CONFORMANCE WITH ACI 318, 11, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 301, 11, "SPECIFICATIONS FOR STRUCTURAL CONCRETE"

ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE. (145 PCF +/-)
ALL CEMENT SHALL CONFORM TO ASTM C150, TYPE 1.
MAXIMUM AGGREGATE SIZE SHALL BE 1 1/2" INCHES FOR FOOTINGS, 3/4" FOR ALL WALLS AND SLABS AND SHALL CONFORM TO ASTM C33

ALL CONCRETE WORK SHALL BE EXECUTED IN STRICT ACCORDANCE WITH SECTIONS 5.7 THRU 5.13 OF ACI 318. THE CONTRACTOR SHALL OBTAIN AND READ THESE SECTIONS OF THE CODE PRIOR TO PLACING CONCRETE.

CONCRETE REINFORCING
DEFORMED BARS ASTM A615, GRADE 60
WELDED WIRE FABRIC ASTM A185
(PROVIDE IN FLAT SHEETS) USE PLASTIC CHAIRS FOR SLAB ON GRADE

REINFORCING PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT EXCEPT AS SHOWN ON THE DRAWINGS OR PERMITTED BY THE EOR.

CONTRACTOR SHALL VERIFY DIMENSIONS AND LOCATIONS OF ALL SLOTS, PIPE SLEEVES, ETC., AS REQUIRED FOR MECHANICAL TRADES BEFORE CONCRETE IS PLACED.

CALCIUM CHLORIDE SHALL NOT BE USED IN ANY FORM.

CONCRETE TESTING

FOUR SETS OF TEST CYLINDERS SHALL BE MADE AND TESTED FOR EACH 50 YARDS OR LESS OF CONCRETE POURED IN ANY DAY FOR EACH DESIGN MIX. TESTS SHALL BE MADE FOR 7 DAYS, TWO AT 28 DAYS AND ONE HELD IN RESERVE. FIELD CURED CYLINDERS SHALL BE CURED UNDER FIELD CONDITIONS IN ACCORDANCE WITH ASTM C31.

THE SAMPLES USED TO FABRICATE TEST SPECIMENS SHALL BE OBTAINED IN ACCORDANCE WITH ASTM C172.

CONCRETE MIX DESIGNS

LOCATION	MIN f'c AT 28 DAYS	MAX W/C RATIO	SLUMP *	% OF AIR ENTRAIN
FOOTINGS, SLAB ON GRADE	3000 PSI	0.50	5"	4.0

* - PLUS / MINUS 1"

MASONRY

ALL MASONRY WORK SHALL BE IN CONFORMANCE WITH THE LATEST EDITION OF "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530) AND THE "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1) OF THE AMERICAN CONCRETE INSTITUTE.

CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT, HOLLOW, LOAD BEARING UNITS CONFORMING TO ASTM C90, TYPE N-II

MORTAR SHALL CONFORM TO ASTM C270, TYPE M OR S.
ALL PORTLAND CEMENT SHALL CONFORM TO ASTM C150 TYPE I.
LIME SHALL CONFORM TO ASTM C207.
ALL MASONRY CEMENT SHALL CONFORM TO ASTM C91.
THICKNESS OF MORTAR SHALL NOT EXCEED 5/8".
FULL BEAD AND HEAD JOINTS SHALL BE USED.

MASONRY GROUT SHALL CONFORM TO ASTM C476.
Fc OF GROUT SHALL BE 3000 PSI MIN.
THE MAXIMUM AGGREGATE SIZE SHALL BE 3/8" GRADED TO PRODUCE FINE GROUT IN CONFORMANCE WITH ASTM C476 AND C404.
SLUMP OF GROUT SHALL BE 8 TO 11 INCHES.

MINIMUM 28 DAY ULTIMATE COMPRESSIVE STRENGTH (fm=1500 PSI)

MASONRY WALL REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60. PROVIDE 48 BAR DIA LAP ON ALL SPLICES.

HORIZONTAL JOINT REINFORCING SHALL BE USED FOR ALL MASONRY CONSTRUCTION, AND SHALL CONSIST OF 9 GAGE, GALVANIZED, LADDER TYPE REINFORCING CONFORMING TO ASTM A82, SPACED AT 16" O.C. VERTICAL MAX. PROVIDE TWO ADDITIONAL COURSES OF REINFORCING ABOVE AND BELOW ALL OPENINGS.

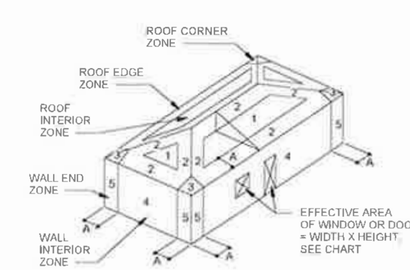
ALL MASONRY WALL WORK SHALL BE GROUTED IN 5' FOOT LIFTS.

PLYWOOD SHEATHING

ALL PLYWOOD SHALL CONFORM TO THE MOST CURRENT APPLICABLE SPECIFICATION AND SUPPLEMENTS OF THE AMERICAN PLYWOOD ASSOCIATION.

ALL PLYWOOD WALL AND ROOF SHEATHING SHALL BE 5/8" INCH DOC PS1 OR PS2, "APA RATED SHEATHING", (WITH A TRADEMARK OF AN APPROVED TESTING AND GRADING AGENCY) 5 PLY, 32/16 SPAN RATING, EXTERIOR GRADE, EXPOSURE 1 GLUE.

SEE PLANS FOR ATTACHMENT SCHEDULES AND DIAGRAMS.



HIPPED ROOFS THETA<3

A=3 FT

TIMBER

ALL WOOD FRAMING CONSTRUCTION, WORKMANSHIP AND MATERIALS (INCLUDING TRUSSES) SHALL CONFORM WITH THE SPECIFICATIONS AND REQUIREMENTS OF THE REFERENCES LISTED BELOW.

"AMERICAN INSTITUTE OF TIMBER CONSTRUCTION"

"NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION"

ALL LUMBER SHALL BE FACTORY MARKED WITH GRADE STAMP OF INSPECTION AGENCY, SHOWING COMPLIANCE WITH GRADING RUL REQM.

GALVANIZED METAL HANGERS AND FRAMING ANCHORS SHALL BE USED AND SHALL BE FASTENED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS.

ANCHORING AND NAILING NOT SPECIFIED SHALL COMPLY WITH THE NAILING SCHEDULE PER THE FBC.

ALL STRUCTURAL LUMBER SHALL BE AS A MINIMUM, NO. 2 GRADE SOUTHERN YELLOW PINE, AND SHALL HAVE AT LEAST THE FOLLOWING MINIMUM ALLOWABLE DESIGN STRESSES AND MODULUS OF ELASTICITY AT A MAXIMUM MOISTURE CONTENT OF 19%

EXTERIOR WALL OPENINGS (CMU/CONC)

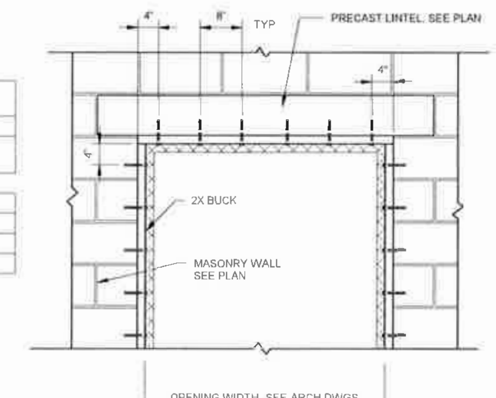
ALL EXTERIOR WALL OPENING ASSEMBLIES SHALL BE DESIGNED TO RESIST THE COMPONENT AND CLADDING WIND LOADS SHOWN ON THESE DRAWINGS.

ALL EXTERIOR WALL OPENING ASSEMBLIES SHALL BE TESTED BY AN APPROVED INDEPENDENT TESTING LABORATORY AND SHALL BE LABELED WITH AN APPROVED LABEL IDENTIFYING THE MANUFACTURER, PERFORMANCE CHARACTERISTICS, AND APPROVED PRODUCT CERTIFYING AGENCY, TESTING LABORATORY, EVALUATION ENTITY OR MIAMI DADE PRODUCT APPROVAL TO INDICATE COMPLIANCE WITH THE REQUIREMENTS OF ONE OF THE FOLLOWING SPECIFICATIONS:
ANSI / AAMA / NWWDA 101 /I.S 2 OR 101/I.S. 2 NAFS OR
AAMA / WDMA / CSA 101 / I.S. 2 / A440 OR
TAS 202 (HVHZ SHALL COMPLY WITH TAS 202 UTILIZING ASTM E1300-02 OR FBC SECTION 2404)

ALL EXTERIOR WALL OPENING ASSEMBLIES SHALL BE ANCHORED TO THE STRUCTURE PER THE MANUFACTURER'S INSTALLATION REQUIREMENTS AND AS A MINIMUM THE REQUIREMENTS SHOWN BELOW. (WHICH EVER IS MORE STRINGENT)

BUCK ANCHORS SHALL BE: 3/8" DIA TITEN SCREWS (SIZED TO PROVIDE 2" MIN PENETRATION INTO CMU JAMB (SIMPSON STRONG TIE)

PROVIDE ANCHORS WITHIN 4" OF ALL CORNERS FOR BOTH VERTICAL AND HORIZONTAL BUCKS, AND AT 8" O.C. FOR THE LENGTH OF EACH BUCK.



COMPONENT AND CLADDING DESIGN WIND PRESSURES FACTORED (ULT)			
ZONE	EFF. AREA (FT2)	POS. PRESS. (PSF)	NEG. PRESS. (PSF)
1	10	22.2	-54.6
1	20	20.8	-53.2
1	50	19.0	-51.4
1	100	17.6	-50.0
2	10	22.2	-91.7
2	20	20.8	-82.0
2	50	19.0	-69.0
2	100	17.6	-59.3
3	10	22.2	-138.0
3	20	20.8	-114.4
3	50	19.0	-82.9
3	100	17.6	-59.3
4	10	50.0	-54.2
4	20	47.7	-51.9
4	50	44.9	-49.1
4	100	42.6	-46.8
5	10	50.0	-66.7
5	20	47.7	-62.0
5	50	44.9	-59.5
5	100	42.6	-51.9

COMPONENT AND CLADDING DESIGN WIND PRESSURES NOMINAL(ASD)			
ZONE	EFF. AREA (FT2)	POS. PRESS. (PSF)	NEG. PRESS. (PSF)
1	10	13.3	-32.8
1	20	12.5	-31.9
1	50	11.4	-30.8
1	100	10.6	-30.0
2	10	13.3	-55.0
2	20	12.5	-49.2
2	50	11.4	-41.4
2	100	10.6	-35.6
3	10	13.3	-82.8
3	20	12.5	-68.6
3	50	11.4	-49.7
3	100	10.6	-35.6
4	10	30.0	-32.5
4	20	28.6	-31.1
4	50	26.9	-29.5
4	100	25.6	-28.1
5	10	30.0	-40.0
5	20	28.6	-37.2
5	50	26.9	-35.7
5	100	25.6	-31.1

POS. PRESSURE ACTS TOWARD THE SURFACE, NEG. PRESSURE ACTS AWAY FROM THE SURFACE

NOTE
FLORIDA BUILDING CODE SECTION 1609.1.5 TESTING TO ALLOWABLE OR NOMINAL LOADS. WHERE TESTING FOR WIND LOAD RESISTANCE IS BASED ON ALLOWABLE OR NOMINAL WIND LOADS, THE DESIGN WIND LOADS DETERMINED IN ACCORDANCE WITH ASCE 7 OR SECTION 1609 ARE PERMITTED TO BE MULTIPLIED BY 0.6 FOR THE PURPOSES OF WIND LOAD RESISTANCE TESTING.

REVISION | DATE | REV#

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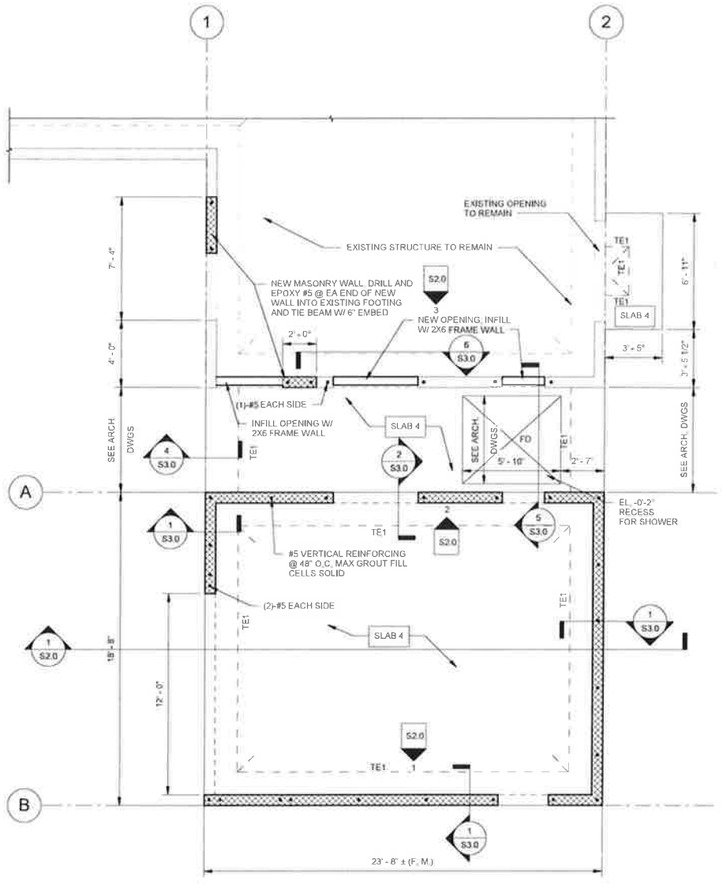
KLUESNER RESIDENCE ADDITION

Enter address here

DRAWING TITLE
STRUCTURAL COVER SHEET & GENERAL NOTES

ENGINEER OF RECORD
THOMAS L. ADAMS, PE
PE #55343

DATE	07/27/16
SCALE	As indicated
PROJ. NO.	2016056
DESIGNED BY	TA
DRAWN BY	CW
CHECKED BY	TA
DRAWING NO.	S0.0



1 FOUNDATION AND FLOOR PLAN

1/4" = 1'-0"

PLAN NOTES

1. ▭ DENOTES EXISTING MASONRY WALLS
2. ▨ DENOTES LOAD BEARING 8" MASONRY WALLS
   ALL WALLS SHALL BE REINFORCED PER PLAN
   ALL WALL OPENINGS SHALL HAVE TYPE L1 LINTEL U.N.O.
3. SEE ARCHITECTURAL PLANS FOR LOCATIONS OF ALL WINDOWS AND DOORS.
4. F. M. - DENOTES FIELD MEASURE

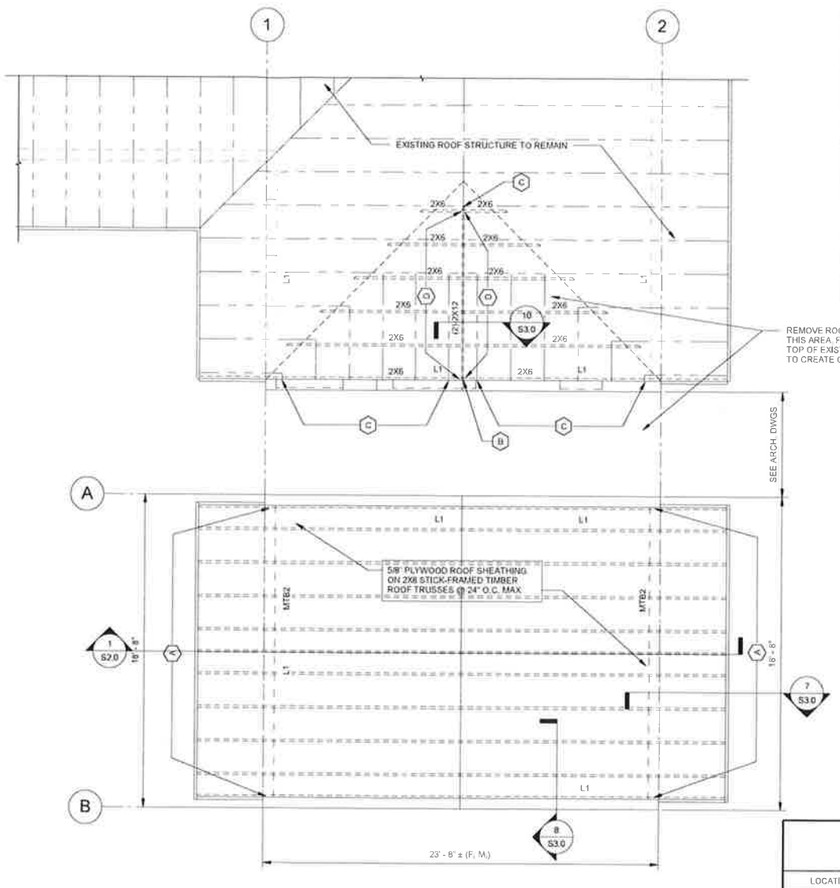
THICKENED SLAB EDGE FOOTING SCHEDULE

MARK	WIDTH X LENGTH	DEPTH	REINFORCEMENT
TE1	1'-4" X CONT	1'-0"	(2)-#5 CONT., BOTT.

SLAB ON GRADE SCHEDULE

MARK	THICKNESS	REINFORCEMENT
SLAB4	4"	6X6 W1.4XW1.4 W.W.F. (OR FIBER MESH)

1. SEE GENERAL NOTES FOR COMPACTION REQUIREMENTS.
2. PROVIDE NEW 10 MIL VAPOR BARRIER, TAPE ALL JOINTS. PENETRATION TEARS CUTS OR DAMAGE PRIOR TO PLACING CONCRETE
3. SUBMIT CONCRETE CURING METHOD TO EOR FOR REVIEW AND APPROVAL PRIOR TO PLACING CONCRETE.



2 ROOF FRAMING PLAN

1/4" = 1'-0"

PLAN NOTES

1. TRUSS PROFILE AND OVERHANG TO MATCH EXISTING. (FIELD VERIFY)
2. T.O. WALL (±8'-0") (MATCH EXISTING)

CASTCRETE LINTEL SCHEDULE

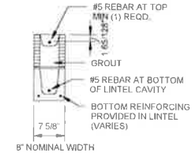
MARK	TYPE	COMMENTS
L1	8F8-1B	PRECAST LINTEL, PROVIDE 8" BEARING EA SIDE

TYPE DESIGNATION

F = FILLED WITH GROUT / U = UNFILLED
QUANTITY OF #5 REBAR AT BOTTOM OF LINTEL CAVITY

8F16-1B/1T

NOMINAL HEIGHT
NOMINAL WIDTH
QUANTITY OF #5 REBAR AT TOP



MASONRY TIE BEAM SCHEDULE

MARK	DIMENSIONS		REINFORCING			
	WIDTH	DEPTH	TOP	MID.	BOTT.	REMARKS
MTB2	7 5/8"	11 1/4"	1-#5	-	1-#5	-

TRUSS ANCHOR SCHEDULE

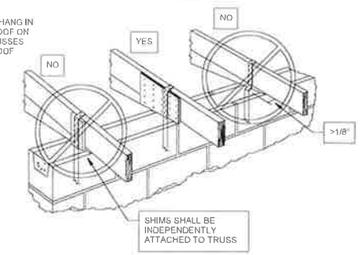
CONN. MARK	SIMPSON MODEL	FASTENER REQUIREMENTS		CAPACITY
		TRUSS	SUPPORT	(LBS)
A	MTSM16	(7)-10d X 1 1/2" NAILS	(4)-1/4" X 2 1/4" TITEN	860
B	HU212-2	(16)-10d X 1 1/2" NAILS	(16)-16d X 1 1/2" NAILS	980
C	H10A-2	(9)-10d X 1 1/2" NAILS	(9)-10d X 1 1/2" NAILS	1245
D	HUS26	(6)-16d X 1 1/2" NAILS	(14)-1/4" X 2 1/4" TITEN	1335

ALL ANCHORS SHALL BE INSTALLED TO MEET THE MANUFACTURER'S INSTALLATION REQUIREMENTS. PROVIDE SHIMS AS REQUIRED TO MEET TOLERANCES.

TRUSS ANCHORS INDICATED ARE PRELIMINARY AND NOT INTENDED TO BE FINAL. ANCHORS SHOWN ARE FOR PRELIMINARY PRICING ONLY. ALL FINAL UPLIFTS SHALL BE VERIFIED WITH APPROVED PRE-ENGINEERED WOOD TRUSS SHOP DRAWINGS PRIOR TO ORDERING MATERIALS OR SETTING OF TRUSS ANCHORS. CONTRACTOR TO COORDINATE FINAL TRUSS CONFIGURATION AND UPLIFT LOADS W/ ANCHORS SELECTED AND SHALL SUBMIT FOR REVIEW AND APPROVAL TO ENGINEER OF RECORD IF ALTERNATE CONNECTORS ARE USED.

SEE TRUSS MANUFACTURER'S DRAWINGS FOR ALL TRUSS TO TRUSS CONNECTIONS

REMOVE ROOF OVERHANG IN THIS AREA. FRAME ROOF ON TOP OF EXISTING TRUSSES TO CREATE GABLE ROOF



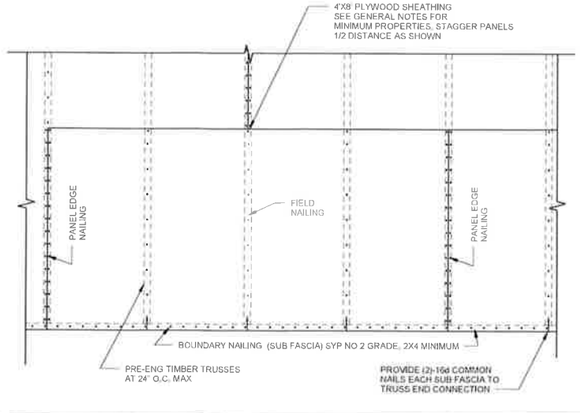
MISLOCATED ANCHORS

MISLOCATED EMBEDDED TRUSS ANCHORS WHICH ARE GREATER THAN 1/8" AND LESS THAN 1 1/2" FROM THE FACE OF THE TRUSS SHALL HAVE A SHIM PROVIDED BETWEEN THE STRAP AND THE TRUSS. ALL SHIMS AND REQUIRED FASTENERS SHALL BE DESIGNED BY THE TRUSS MANUFACTURER'S DELEGATED ENGINEER.

PLYWOOD ROOF SHEATHING NAILING SCHEDULE

LOCATION	NAIL SIZE	SPACING
BOUNDARY	8d	3"
PANEL EDGE	8d	3"
FIELD	8d	6"

MINIMUM NAIL PROPERTIES: 8d RING SHANK NAILS
0.113" NOMINAL SHANK DIAMETER
0.012" RING DIAMETER
10-20 RINGS PER INCH
0.28" FULL ROUND HEAD DIAMETER
2" MINIMUM NAIL LENGTH



REVISION

REV	DATE

CONSTRUCTION ENGINEERING GROUP
consulting engineers
1631 west roy gallo, suite a
melbourne, fl 32935
tel 321.253.1721
fax 321.253.1722
www.cegengineering.com
license #0026797

KLUESNER RESIDENCE ADDITION

Enter address here

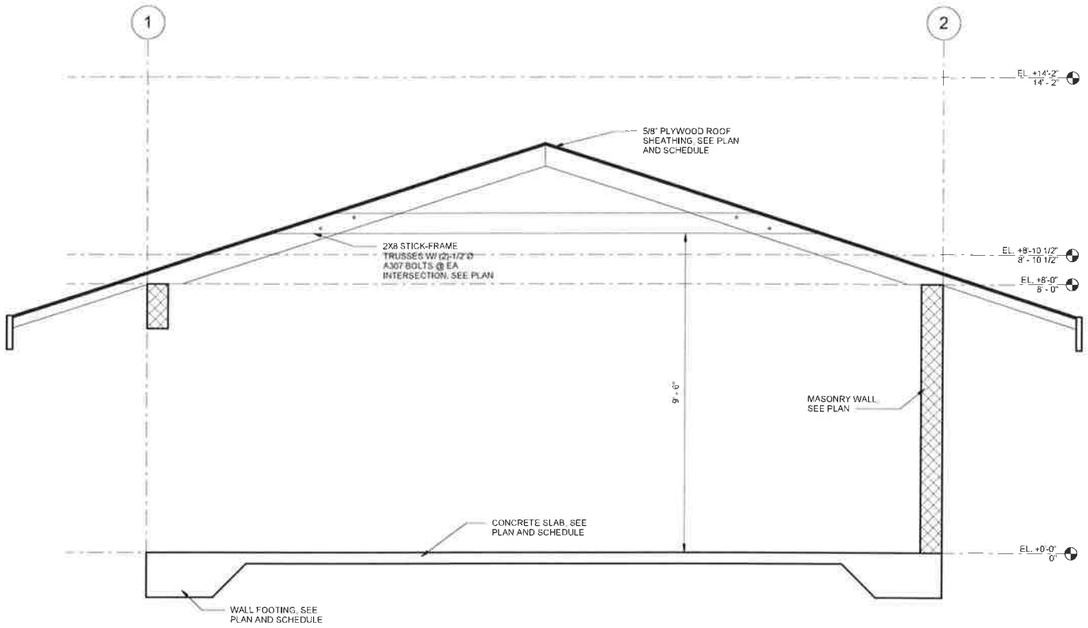
DRAWING TITLE
PLANS AND SCHEDULES

ENGINEER OF RECORD

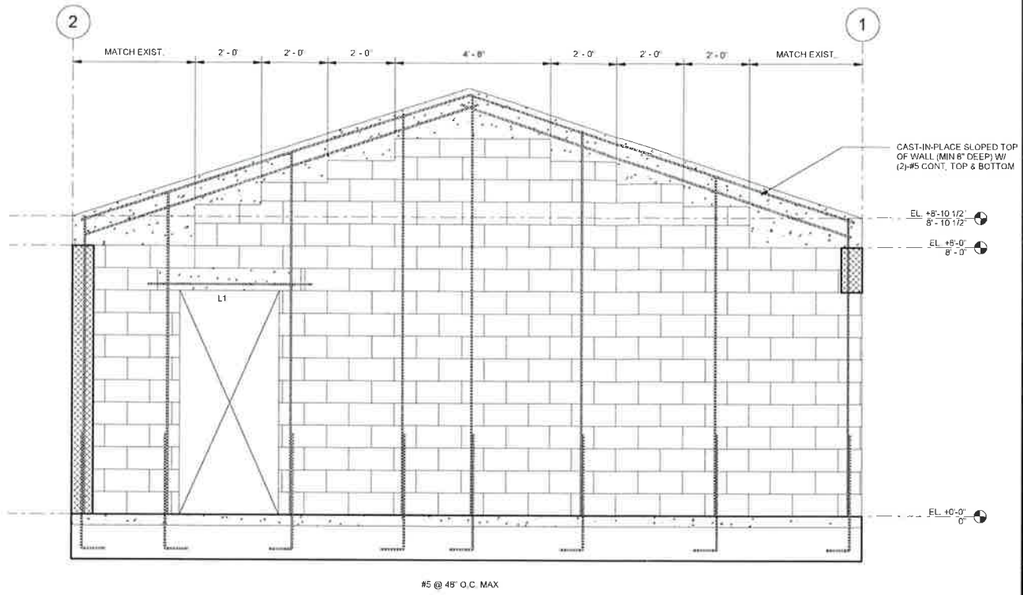
THOMAS L. ADAMS LICENSE No. 55343 STATE OF FLORIDA PROFESSIONAL ENGINEER

THOMAS L. ADAMS, PE
PE #55343

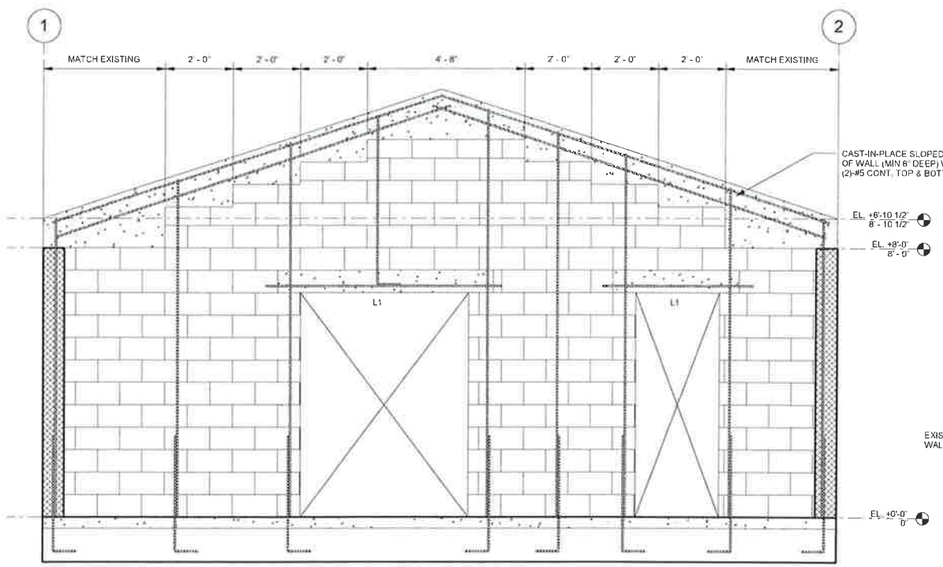
DATE: 07/27/16
SCALE: As indicated
PROJ. NO.: 2016056
DESIGNED BY: TA
DRAWN BY: CW
CHECKED BY: TA
DRAWING NO.: S1.0



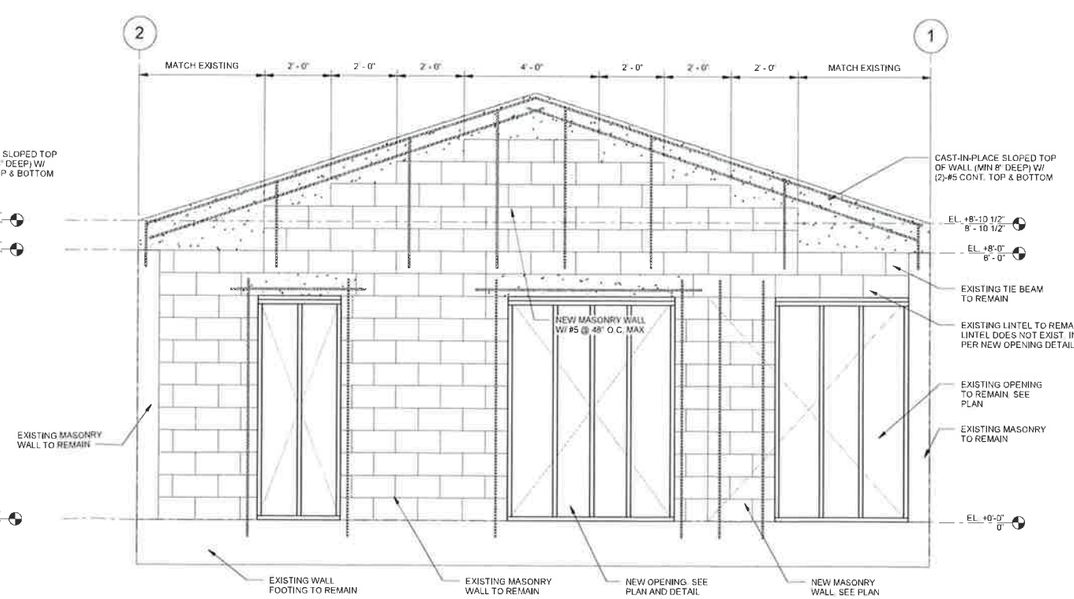
1 BUILDING SECTION
1/2" = 1'-0"



2 WALL ELEVATION
1/2" = 1'-0"



3 WALL ELEVATION
1/2" = 1'-0"



4 WALL ELEVATION
1/2" = 1'-0"

REVISION | DATE | REV#

CONSTRUCTION ENGINEERING GROUP
consulting engineers
7451 west congaree, suite a
melbourne, fl. 32935
tel 321.253.1721
fax 321.253.1721
www.cegengineering.com
license: #0068097

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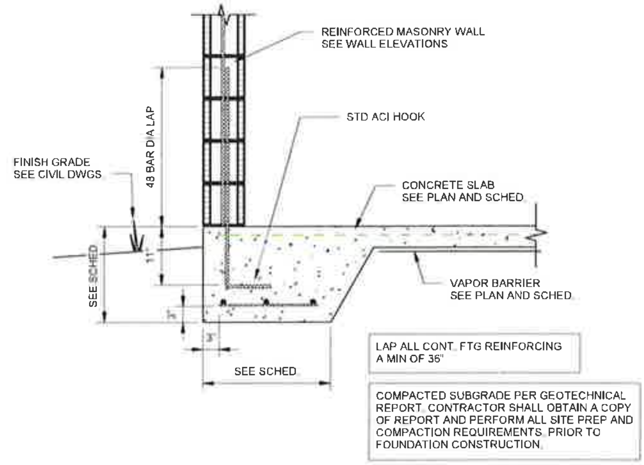
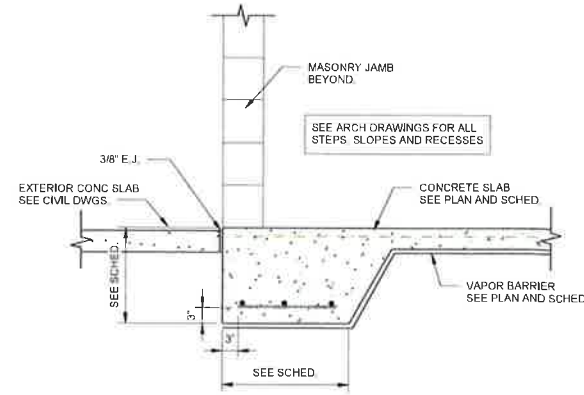
DRAWING TITLE
WALL ELEVATIONS

ENGINEER OF RECORD
THOMAS L. ADAMS, PE
PE #55343

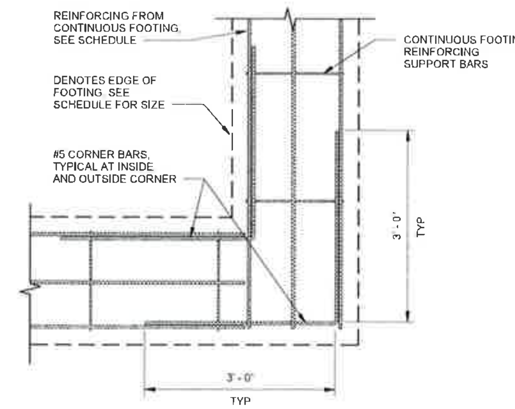
DATE	07/27/16
SCALE	1/2" = 1'-0"
PROJ. NO.	2016056
DESIGNED BY	TA
DRAWN BY	CW
CHECKED BY	TA
DRAWING NO.	S2.0



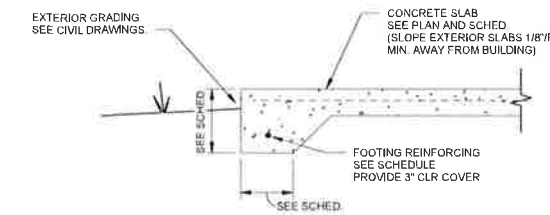
1 SECTION
3/4" = 1'-0"



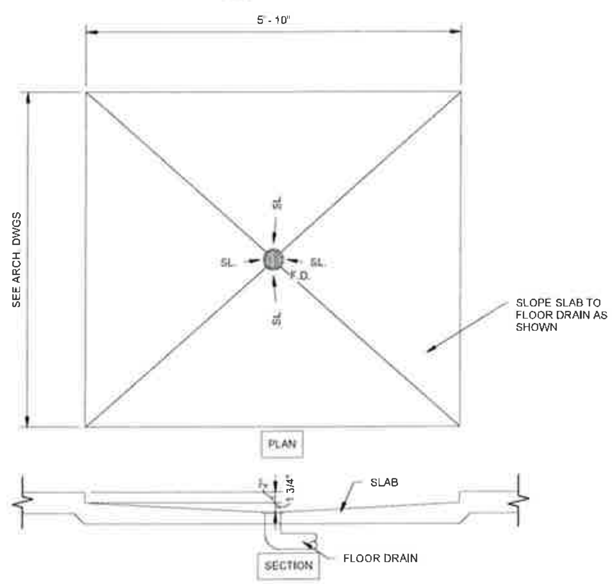
2 SECTION
3/4" = 1'-0"



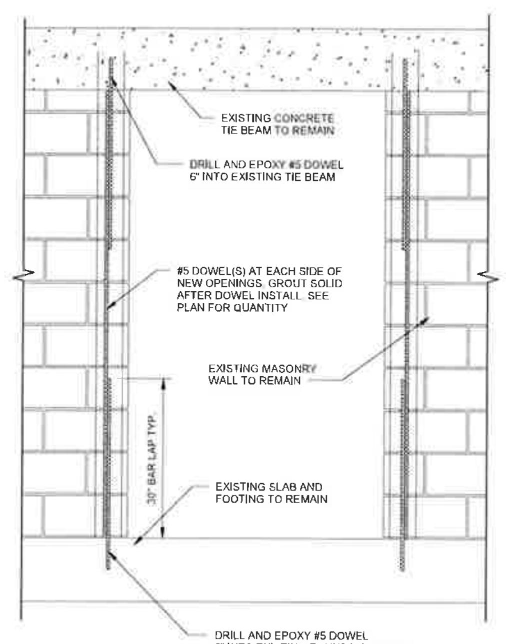
3 SECTION
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"

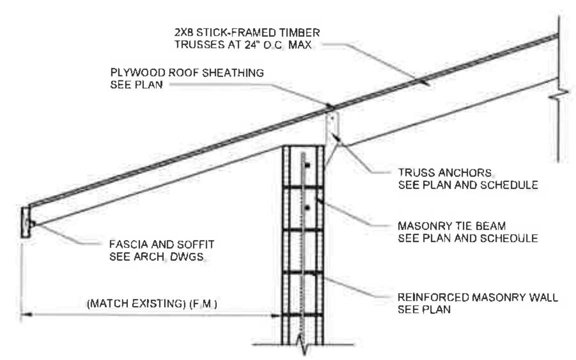


5 SECTION
3/4" = 1'-0"

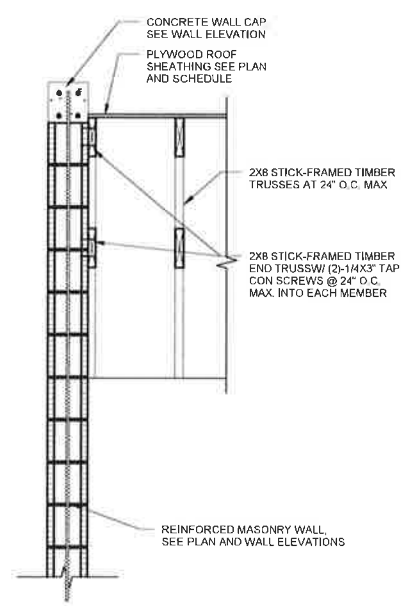


NOTES:
1. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS OF ALL OPENINGS
2. ALL WALL REINFORCING AND NEW LINTELS TO BE INSTALLED PRIOR TO REMOVING CMU.

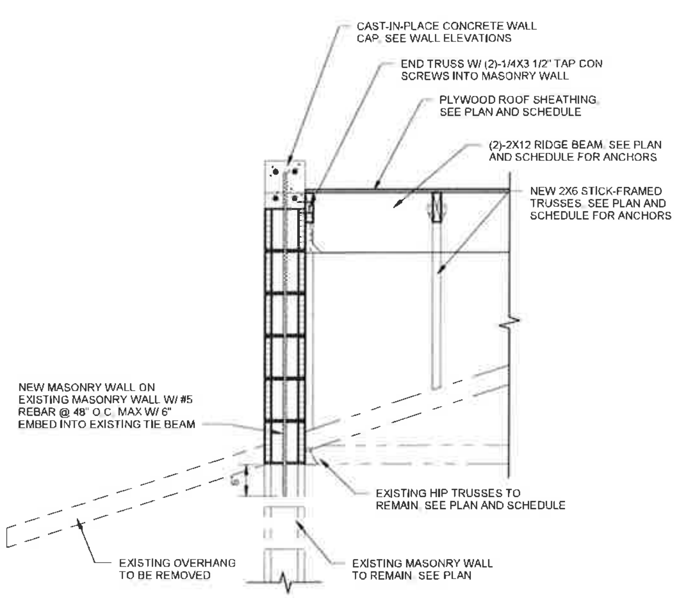
6 SECTION
3/4" = 1'-0"



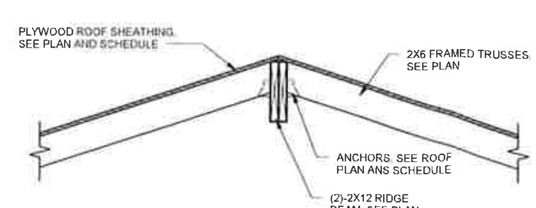
7 SECTION
3/4" = 1'-0"



8 SECTION
3/4" = 1'-0"



9 SECTION
3/4" = 1'-0"



10 SECTION
3/4" = 1'-0"

REV#	DATE	REVISION

CONSTRUCTION ENGINEERING GROUP
consulting engineers

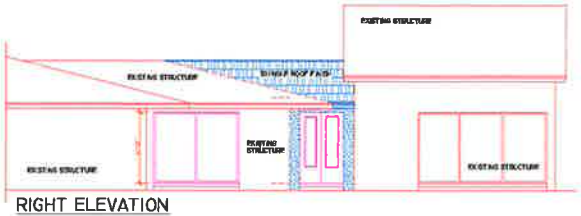
KLUESNER RESIDENCE ADDITION
Enter address here

DRAWING TITLE
SECTIONS AND DETAILS

ENGINEER OF RECORD
THOMAS L. ADAMS LICENSE No 55343 STATE OF FLORIDA PROFESSIONAL ENGINEER
THOMAS L. ADAMS, PE
PE #55343

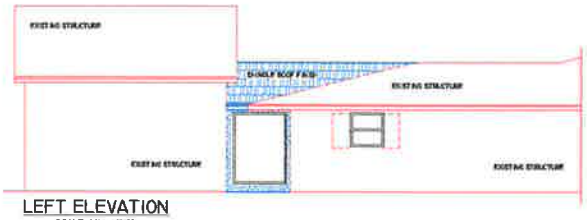
DATE	07/27/16
SCALE	3/4" = 1'-0"
PROJ. NO.	2016056
DESIGNED BY	TA
DRAWN BY	CW
CHECKED BY	TA
DRAWING NO.	S3.0

21



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"

REAR ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"

DESIGN CRITERIA	
FULLY ENCLOSED	
WIND SPEED	130 MPH
EXPOSURE	D
RISK CATEGORY	TWO (II)

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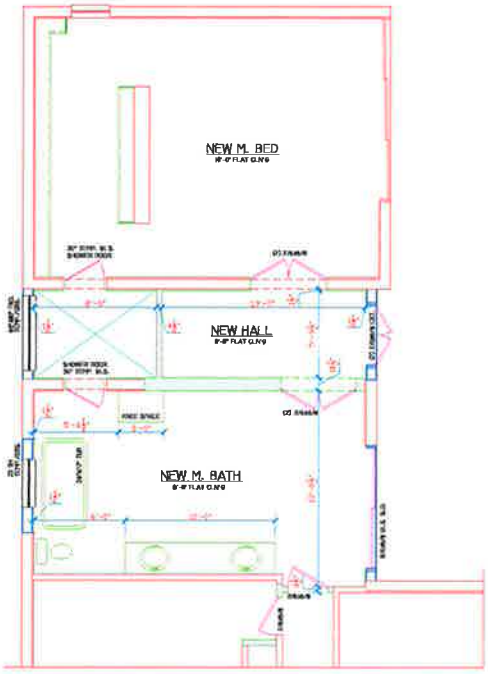
SHEET 1 OF *



FLOOR PLAN

SCALE: 1/4" = 1'-0"

AREA TABULATION (5-20-22)	
NEW LIVING	141 SQ.FT.
EXISTING LIVING	1998 SQ.FT.
EXISTING DETACHED LIVING	469 SQ.FT.
TOTAL LIVING	2698 SQ.FT.

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